



South Florida's Response to Predicted Sea Level Rise

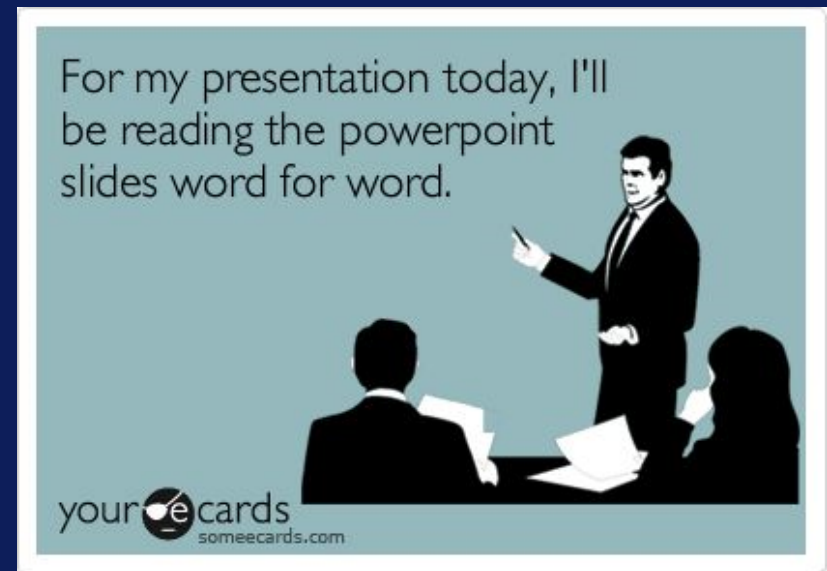
S-29 Forward Pump Station (North Miami Beach)

Garrett Litteken, P.E., CFM

Special Mention: Brian Wozniak, P.E., CFM

Presentation Outline

- Project Background
- Project Scope
 - 2-D Modeling Approach (Original)
- Pump Station Options
- Preliminary Results
- Hydrographs and Boundaries
 - Project Delays
- Expanded Scope
- Variables (Complex)
- HEC-RAS 2D
- Results and Conclusions
- Upcoming Scope



Unique forward pump stations will boost coastal structures' resiliency for future sea level rise

October 18, 2022



Studies suggest that sea levels will rise by a meter or more by the year 2100, threatening trillions of dollars' worth of assets. Long-term sea level rise is a critical threat to coastal areas and poses severe risks and challenges over the upcoming decades.

South Florida may face the adverse effects of sea level rise in the future, and without adaptation investments, the area's annual flood losses could exceed \$25 billion by 2050. The South Florida Water Management District (SFWMD) has been making efforts to address these issues and has prepared a sea level rise and flood resiliency plan.



SFWMD has chosen two pilot locations to evaluate these first-of-their-kind forward pump stations. A team effort including Kimley-Horn and Hanson is designing one of these pilot locations at a gated structure in North Miami Beach, Florida. As part of the preliminary design, Hanson is developing a detailed 2D model to define a broad range of hydraulics that influence conditions leading to the pump intake and the discharge to the canal from a concentrated, high-velocity flow through the structure.

This thorough hydraulic model will provide a comprehensive look at how the flood control infrastructure will handle rising sea levels. Hanson is evaluating the gated structure's capacity and a proposed pump station under current conditions and with the consideration of 1 foot to 3 feet of sea level rise. The model's results will help determine the hydraulic transitions of the proposed intake and outflow channels. The 2D hydraulic model will be used in the project's design to support the inflow assumptions for 3D computational fluid dynamics and in physical models to evaluate more detailed hydraulics within the intake bays and pump station.

Learn about increasing the resiliency of coastal structures from Garrett Litteken at glitteken@hanson-inc.com and Brian Wozniak at bwozniak@hanson-inc.com.



Direct Connection to Tide



S-29 Gates



C-9 Canal



<https://www.climate.gov/maps-data/dataset/sea-level-rise-map-viewer>



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Sea Level Rise – Map Viewer

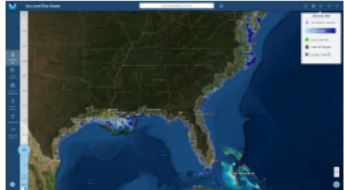
Sea Level Rise and Coastal Flooding Impacts

NOAA's Sea Level Rise map viewer gives users a way to visualize community-level impacts from coastal flooding or sea level rise (up to 10 feet above average high tides). Photo simulations of how future flooding might impact local landmarks are also provided, as well as data related to water depth, connectivity, flood frequency, socio-economic vulnerability, wetland loss and migration, and mapping confidence. The viewer shows areas along the contiguous United States coast, except for the Great Lakes.

Where do these data come from?

The maps are produced using detailed elevation maps with local and regional tidal variability.

EXAMPLE IMAGE



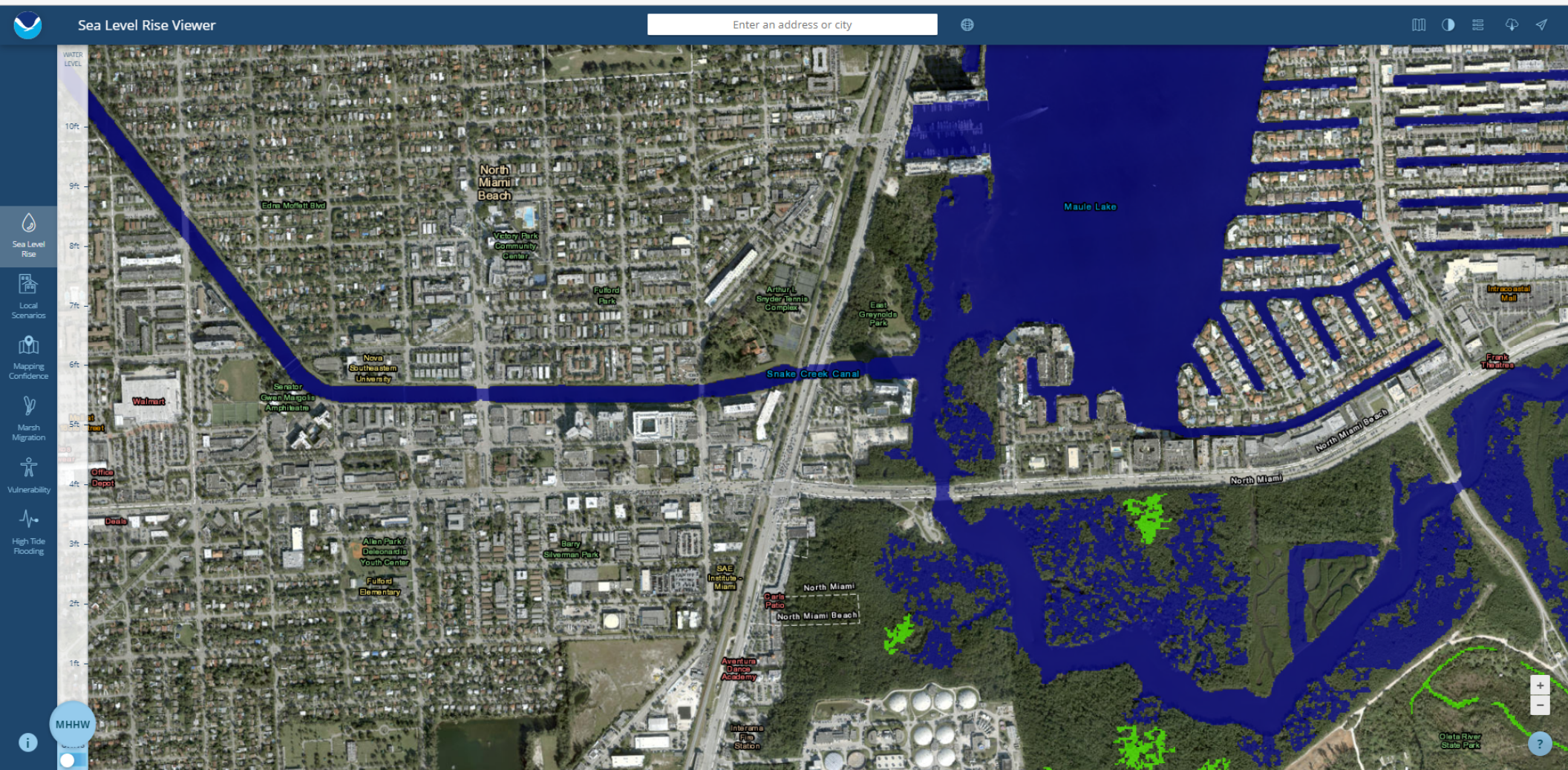
 Click to see more detail

DIRECT LINK

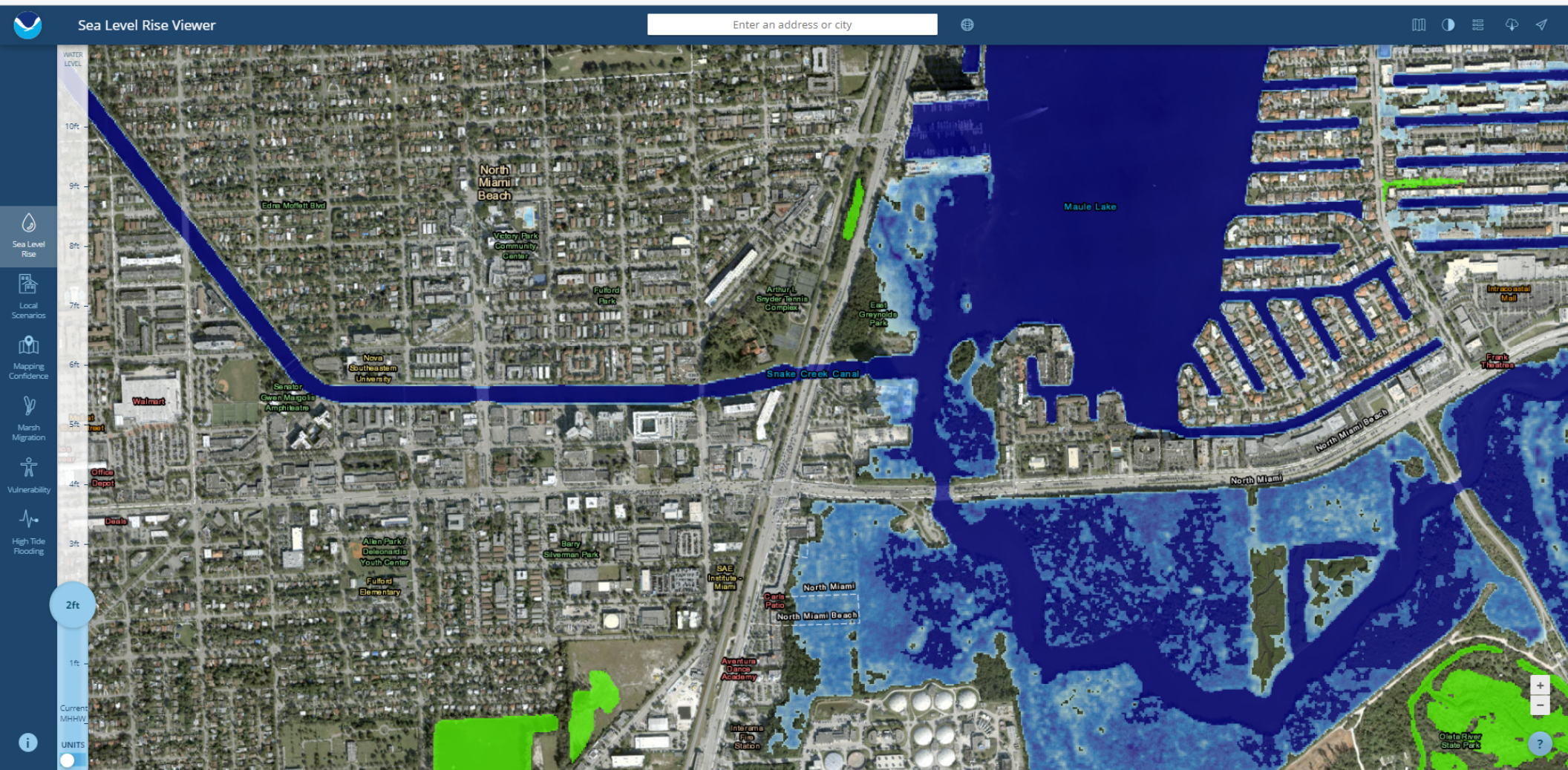
[Sea Level Rise Map Viewer](#)

COVERAGE

US

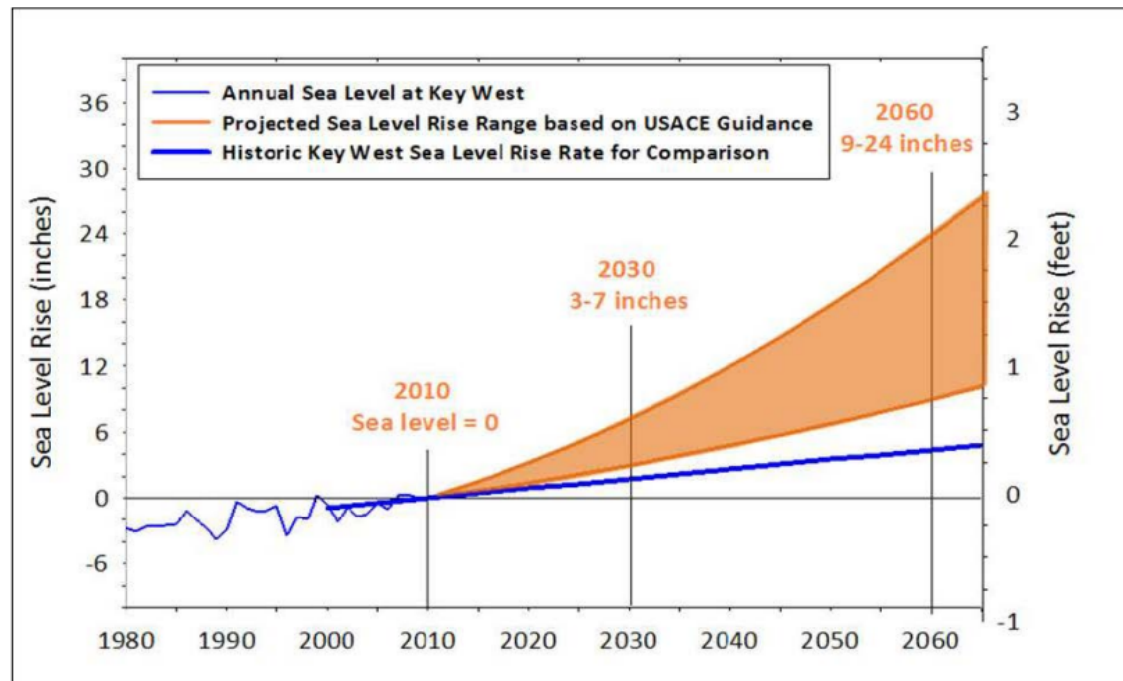








Sea Level Rise Projection



Developed by the Southeast Florida Regional Climate Compact counties and partners, this projection is based on historic tidal information from Key West. It was calculated using U.S. Army Corps of Engineers Guidance Document intermediate and high curves to represent the lower and upper bound for projected sea level rise in Southeast Florida. The rate of sea level rise from Key West over the period of 1913 to 1999 is extrapolated to show how the historic rate compares to projected rates.

Water Management System

- 2,060 miles of canals
- 2,028 miles of levees
- 160 major drainage basins
- 1,413 water control **structures**
- 71 pumping stations
- 60,000 acres of regional wetland Stormwater Treatment Areas
- Lake Okeechobee
 - 450,000 acres water storage area
- Water Conservation Areas
 - 959,000 acre water storage

sfwmd.gov



Presenter: Akintunde Owosina

History How the Program Came About

Aging Infrastructure

- C&SF Project designed and built 60+ years ago
- Approaching end of design life

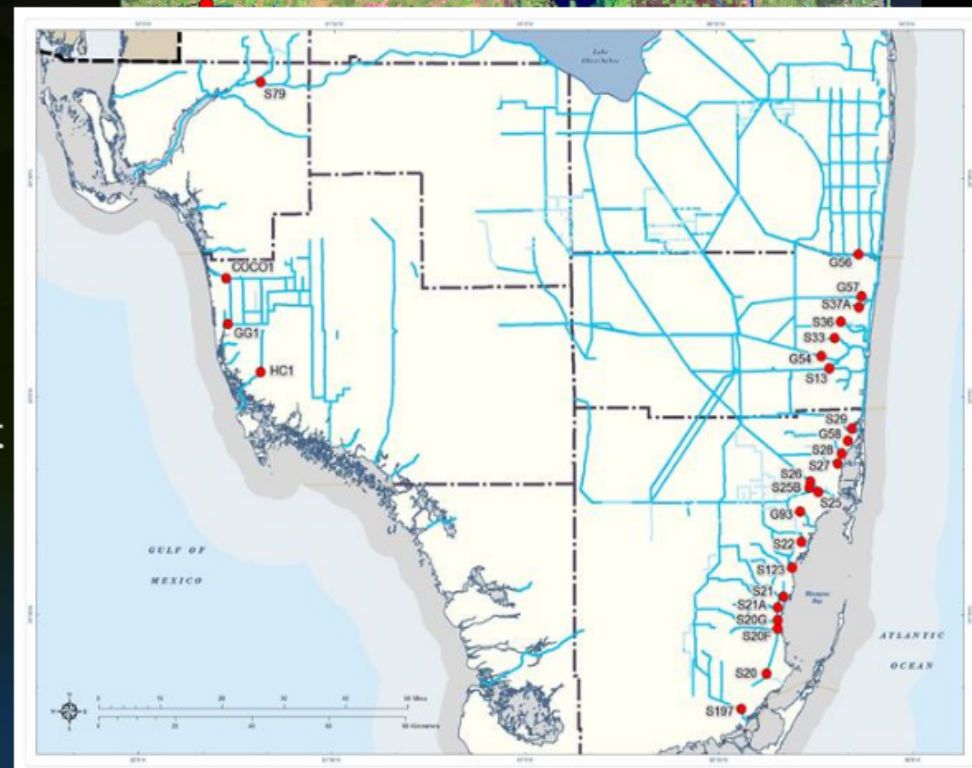
Obsolete Assumptions

- Original design did not account for the sea level rise
- Original design for a population of 2 million people
- Original projections were for less urban development than has occurred over the years

Vulnerable to SLR

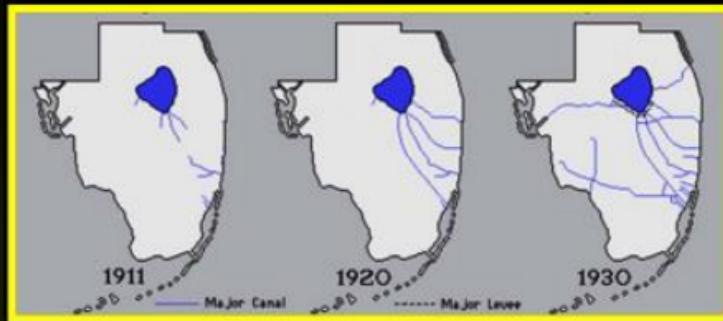
- Several low-lying structures determined to be vulnerable to SLR

Low-lying Tidal Structure Assessment

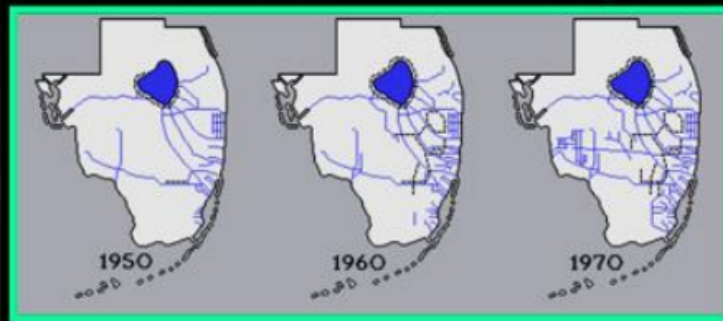


Recognizing Changed Conditions

Pre-1948 Drainage Projects

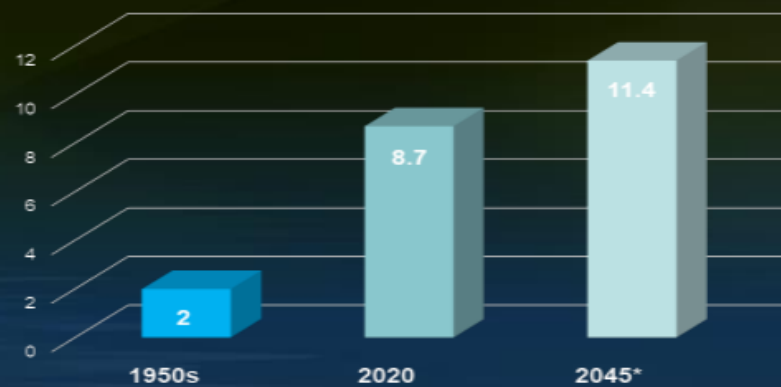


Post-1948 C & S Florida Project



LAND DEVELOPMENT & POPULATION GROWTH

Population (million)



* Estimate taken from BEBR 2017 publication (Median, SFWMD boundaries)

Presenter: Carolina Maran 9

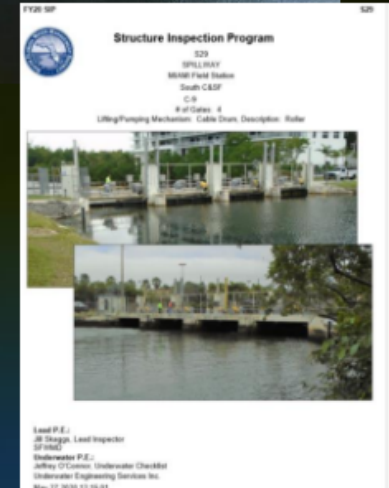
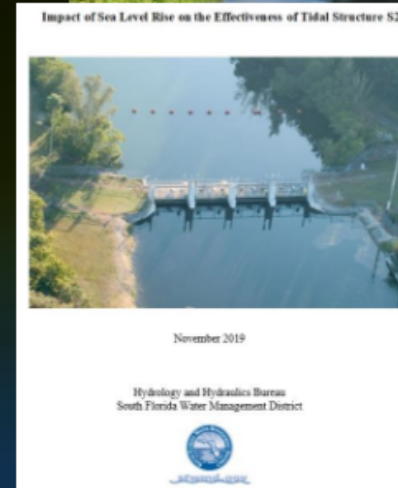
Current Limitations in C&SF Operation

Reduction in Discharge Capacity as a Result of SLR



S29 Coastal Structure Resiliency

- Benefiting Broward & Miami Dade Counties
 - C9 Basin: fully developed, primarily residential and commercial uses, 450K people, 100 square miles
- Enhancing Coastal Structure (elevating gates and other equipment)
- Forward pump (2000cfs) and back up generator
- Flood Barrier (tie back to higher land)
- Real Estate Needs
- Currently in Design



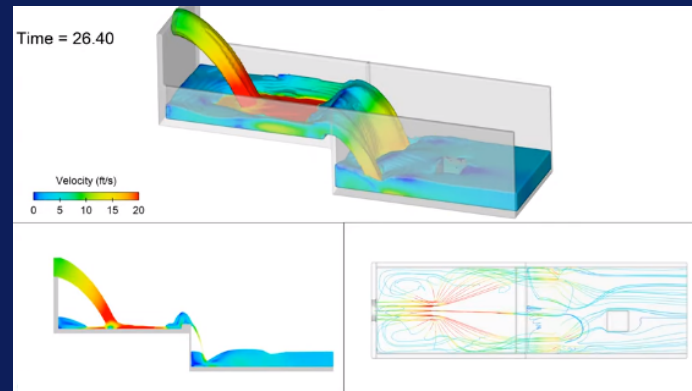
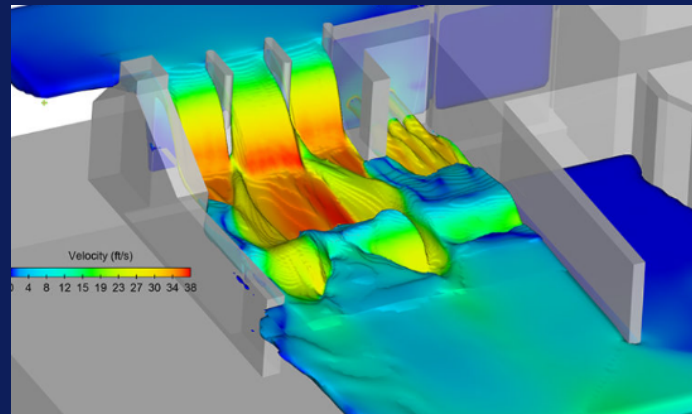
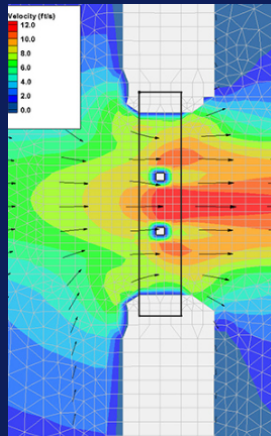
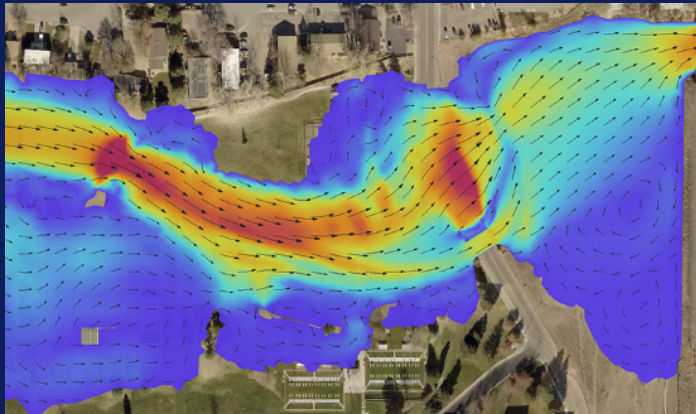
2D (x-y)



Project Scope
3D (CFD) (x-y-z)



Physical

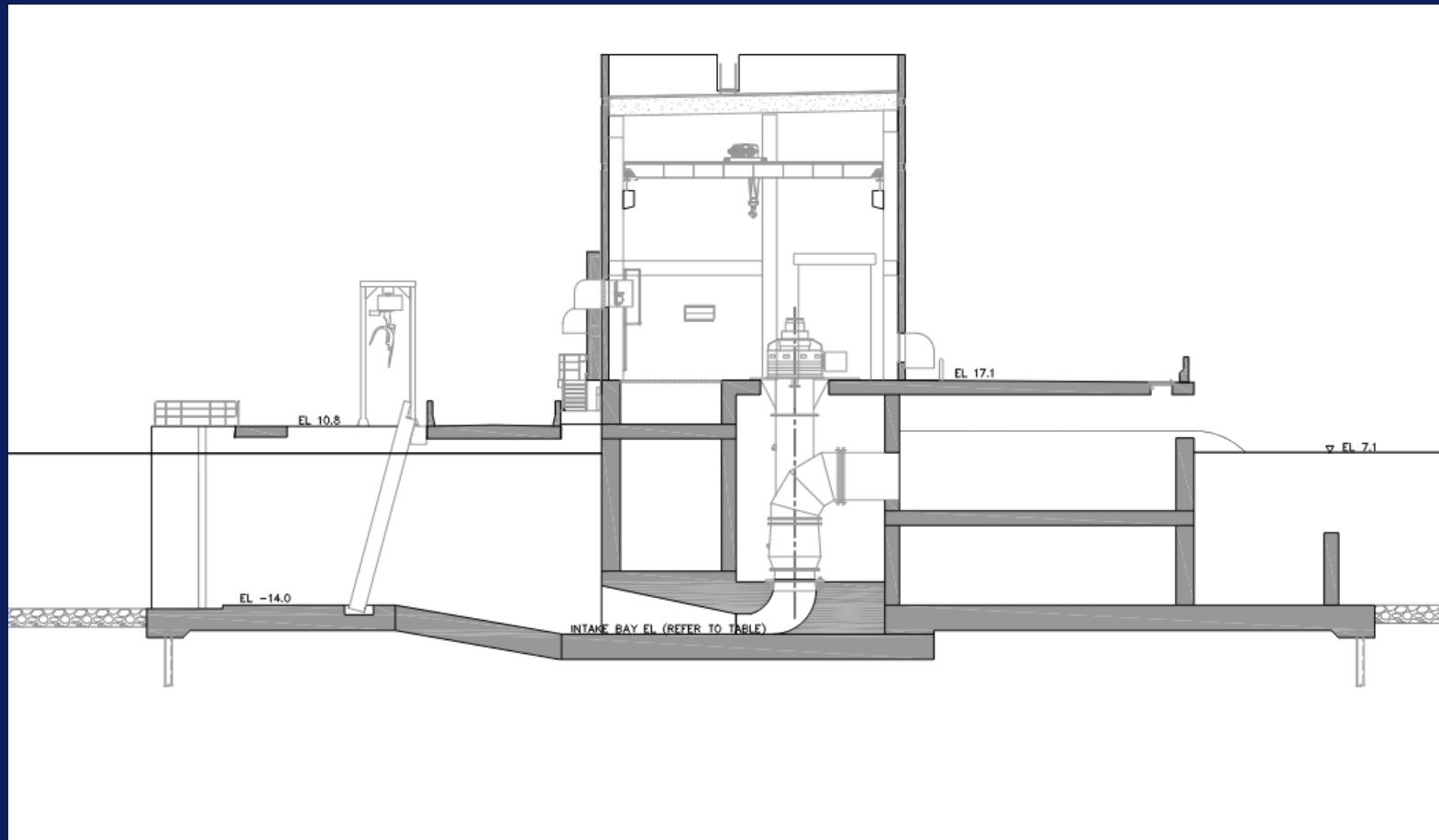


C-9 Canal and S-29 Gates

- C-9 Canal
 - Existing Gates
 - Saltwater Intrusion
- S-29 Pump Station
 - Flood Protection



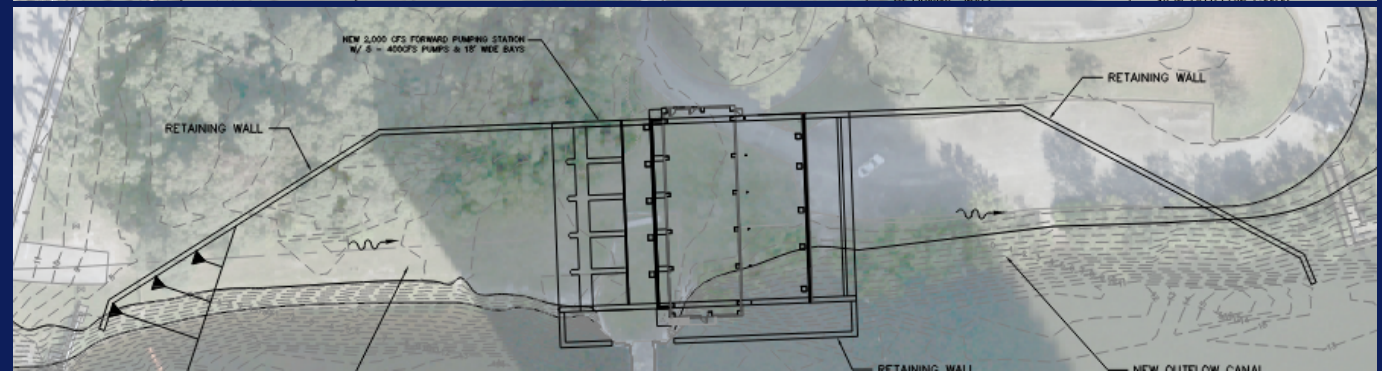
Preliminary Design for Modeling



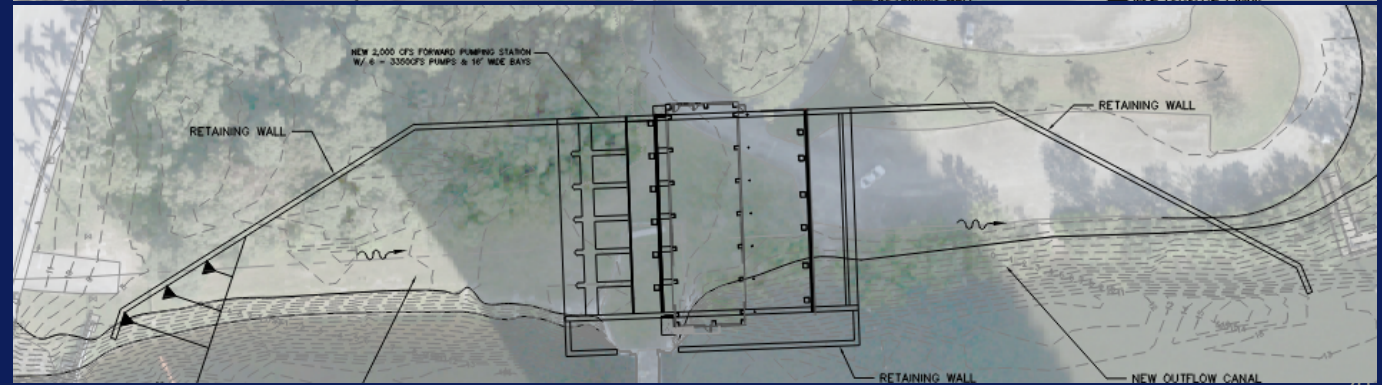
4 Pump Bays – 500-cfs pumps



5 Pump Bays – 400-cfs pumps



6 Pump Bays – 335-cfs pumps



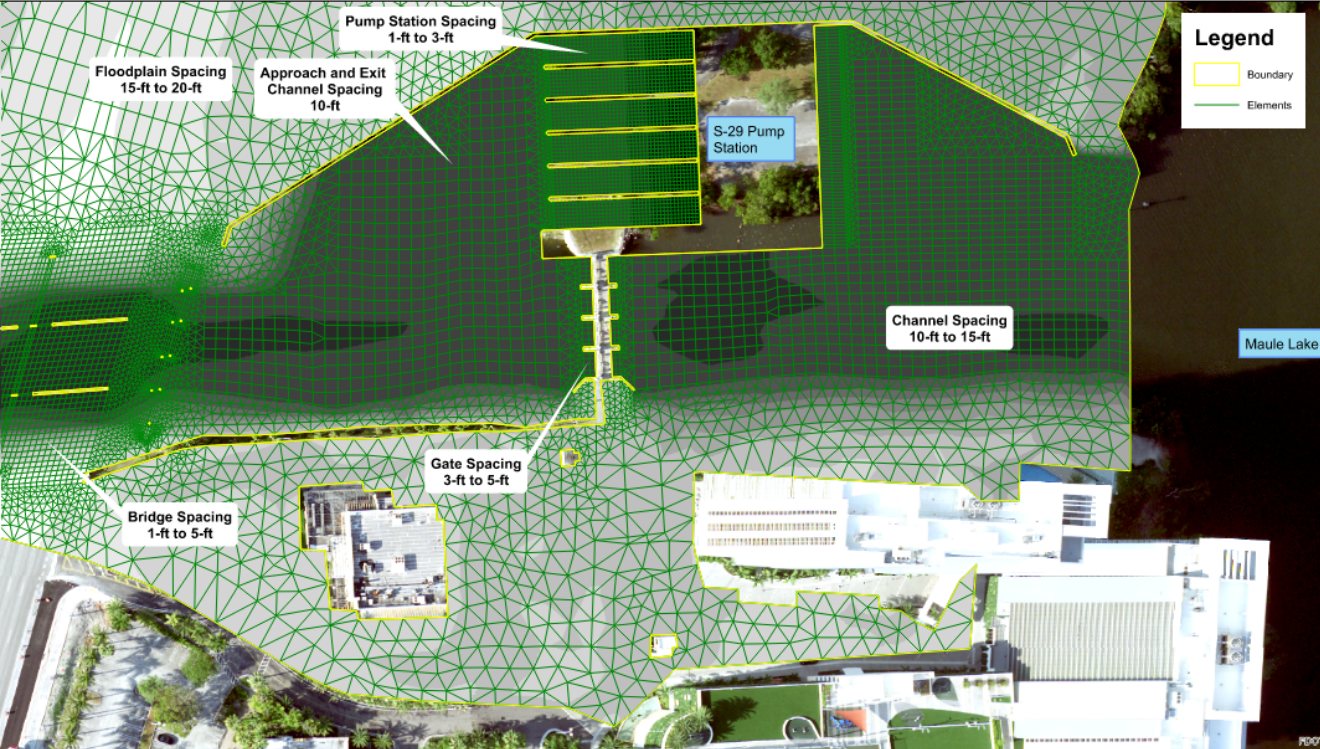
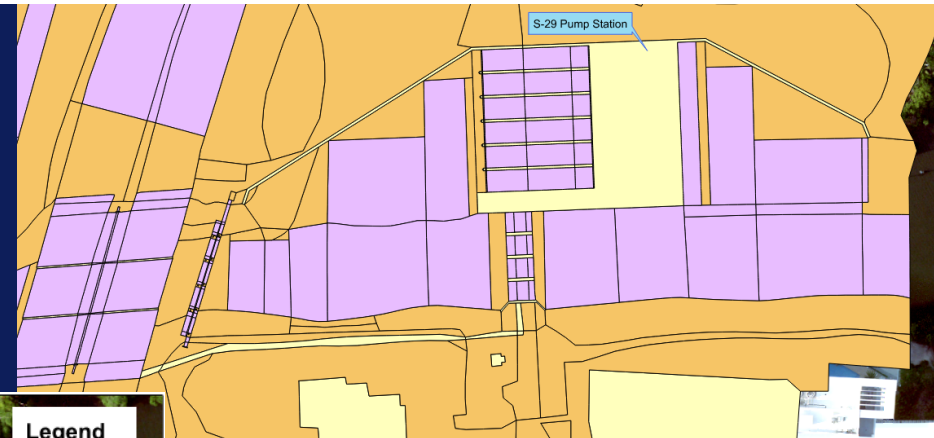
Project Scope – 2D Model

- Develop 2-D Model
 - Evaluate layout for horizontal eddies and optimize and evaluate bridges and canal impacts leading into/out of proposed pump location
- Model Limits
 - 2100 ft upstream S-29
 - 1400 ft downstream S-29



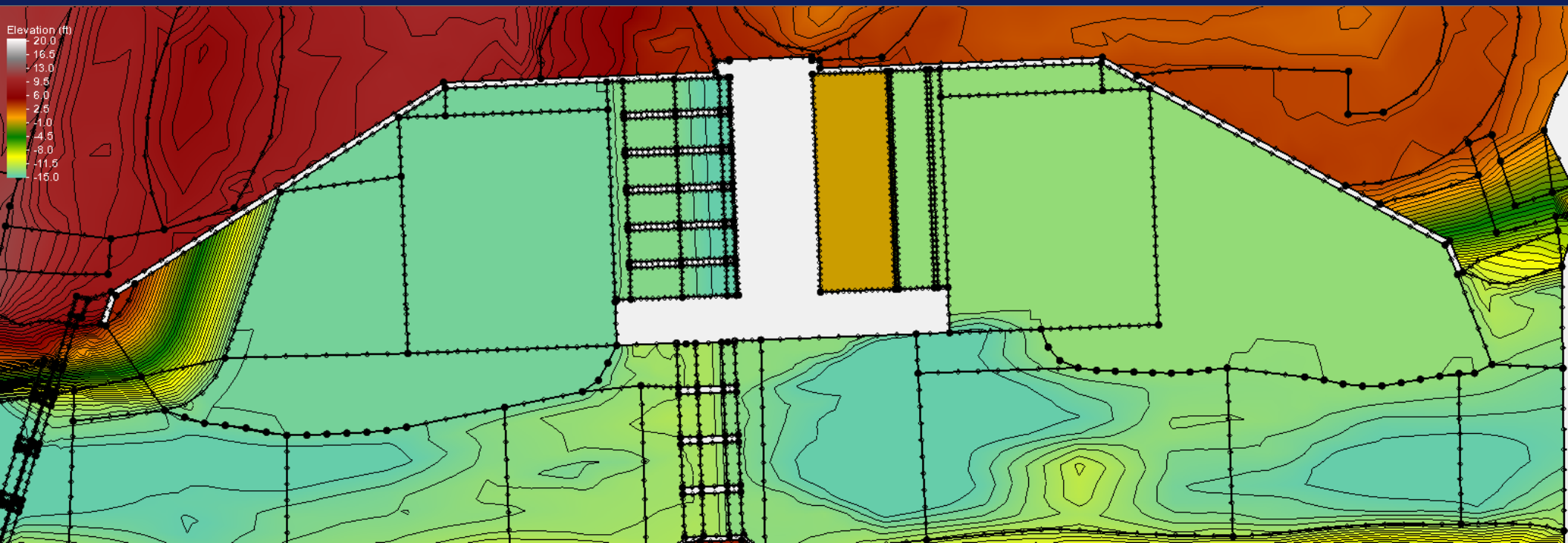
SRH-2D Model Documentation

Computational Node and Elements	
Number of Elements	35124
Number of Nodes	23142
Element Type	linear
Number of Triangular Elements	27653
Number of Quadrilateral Elements	7471

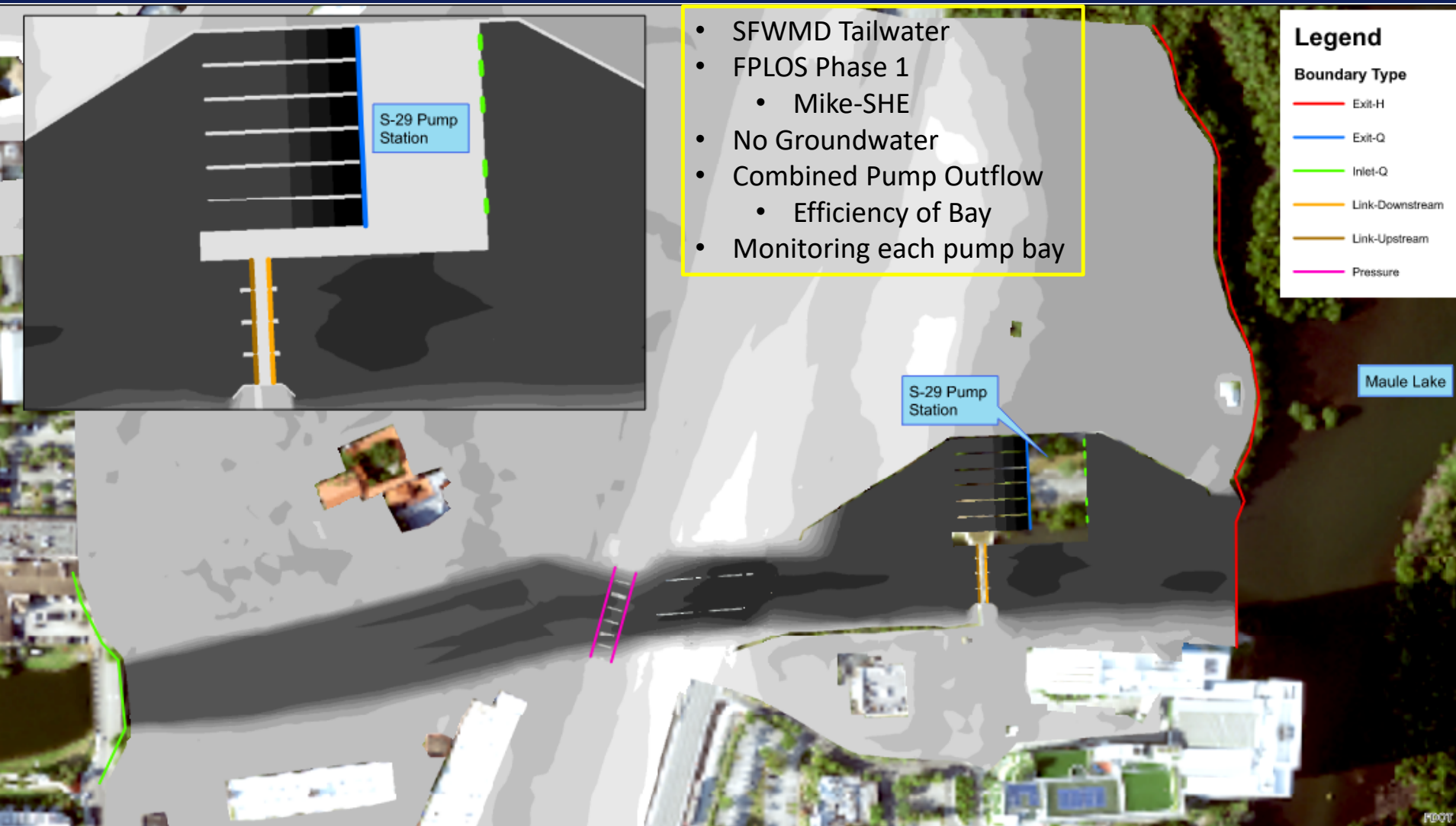


SRH-2D

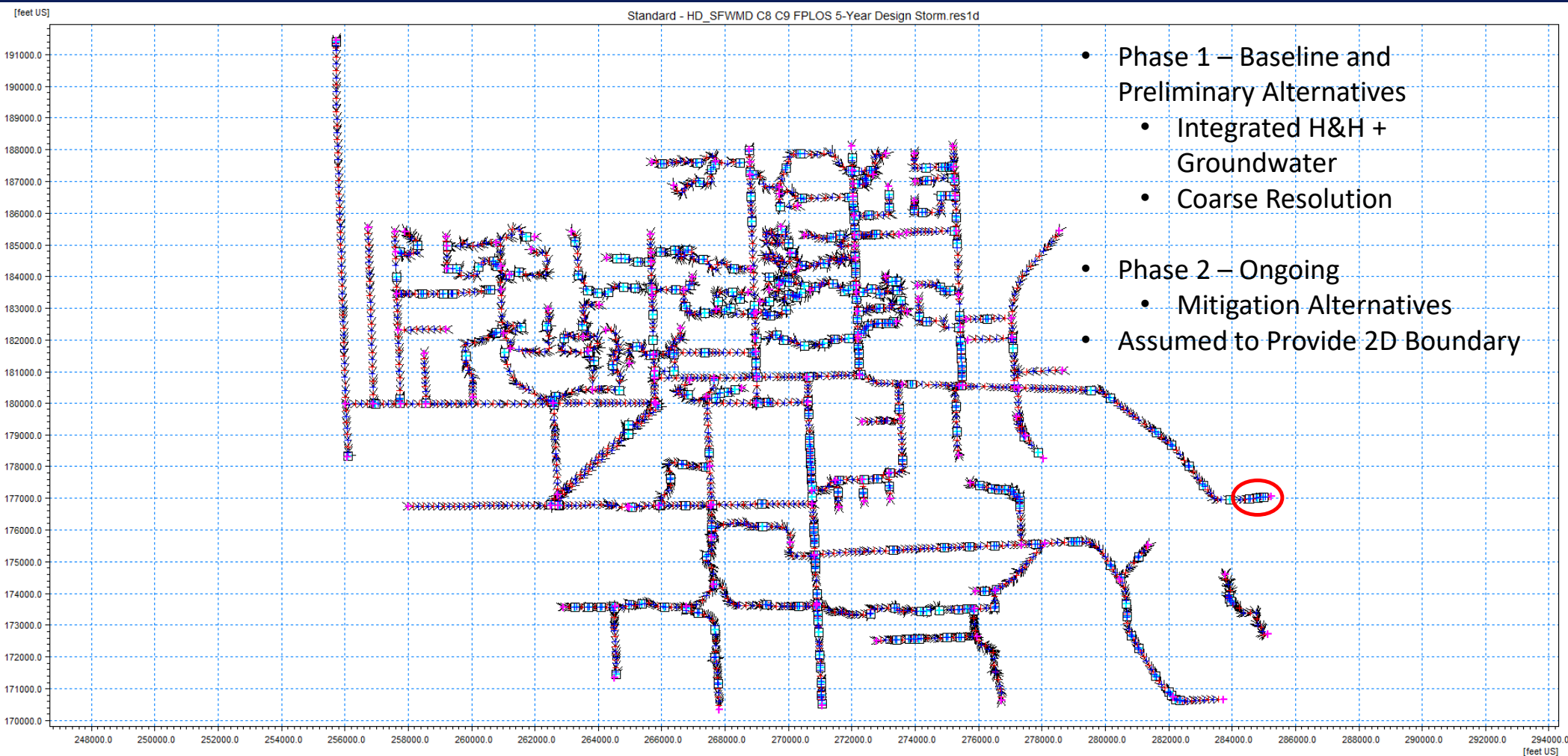
- Each option requires unique mesh, terrain, and boundary conditions



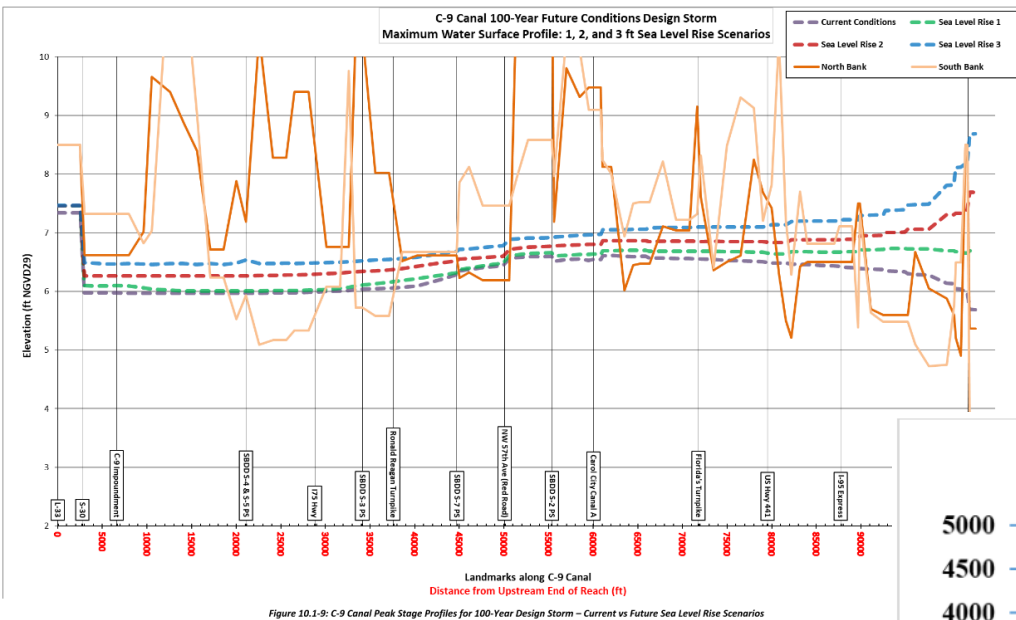
Boundary Conditions



Mike-She & FPLOS



FPLOS Boundary Conditions



- 5-year thru 100-year
 - Current thru SLR3
 - 16 runs per alternative

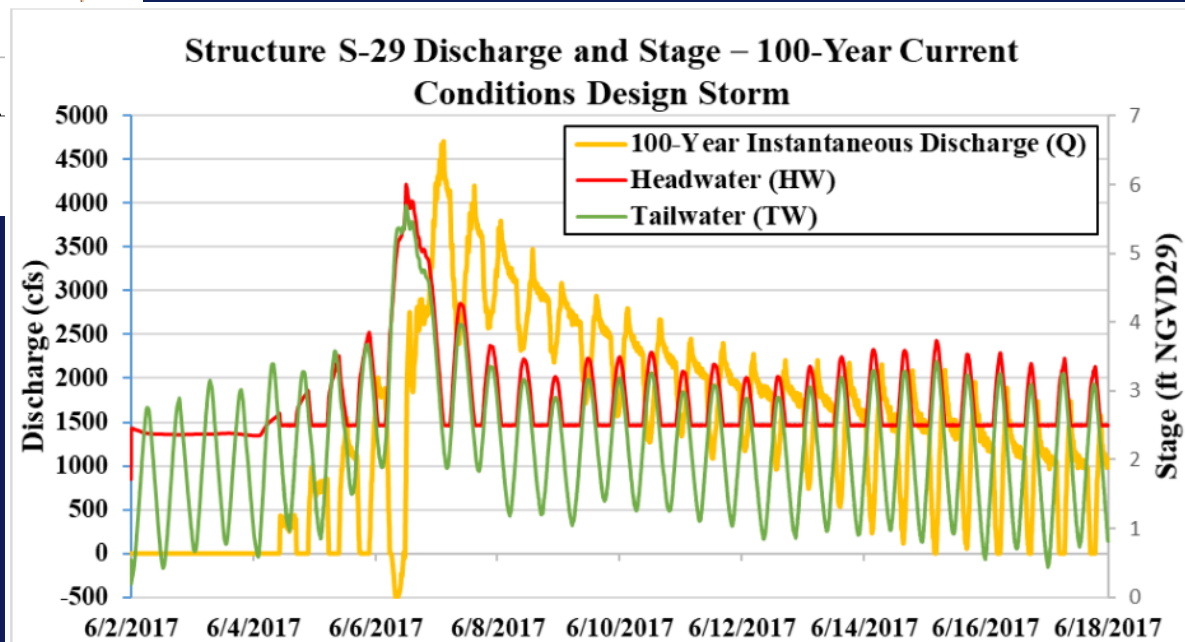
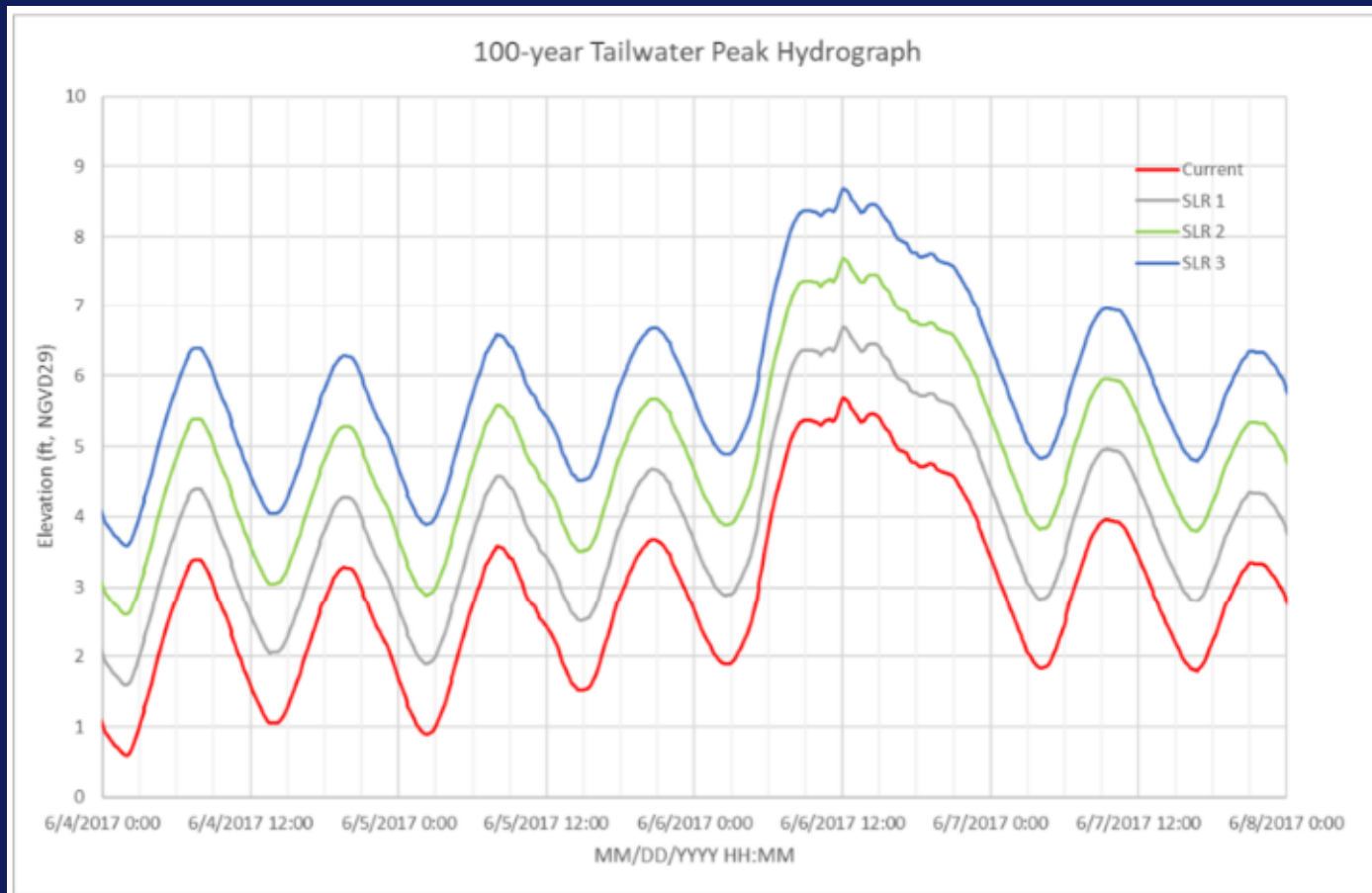
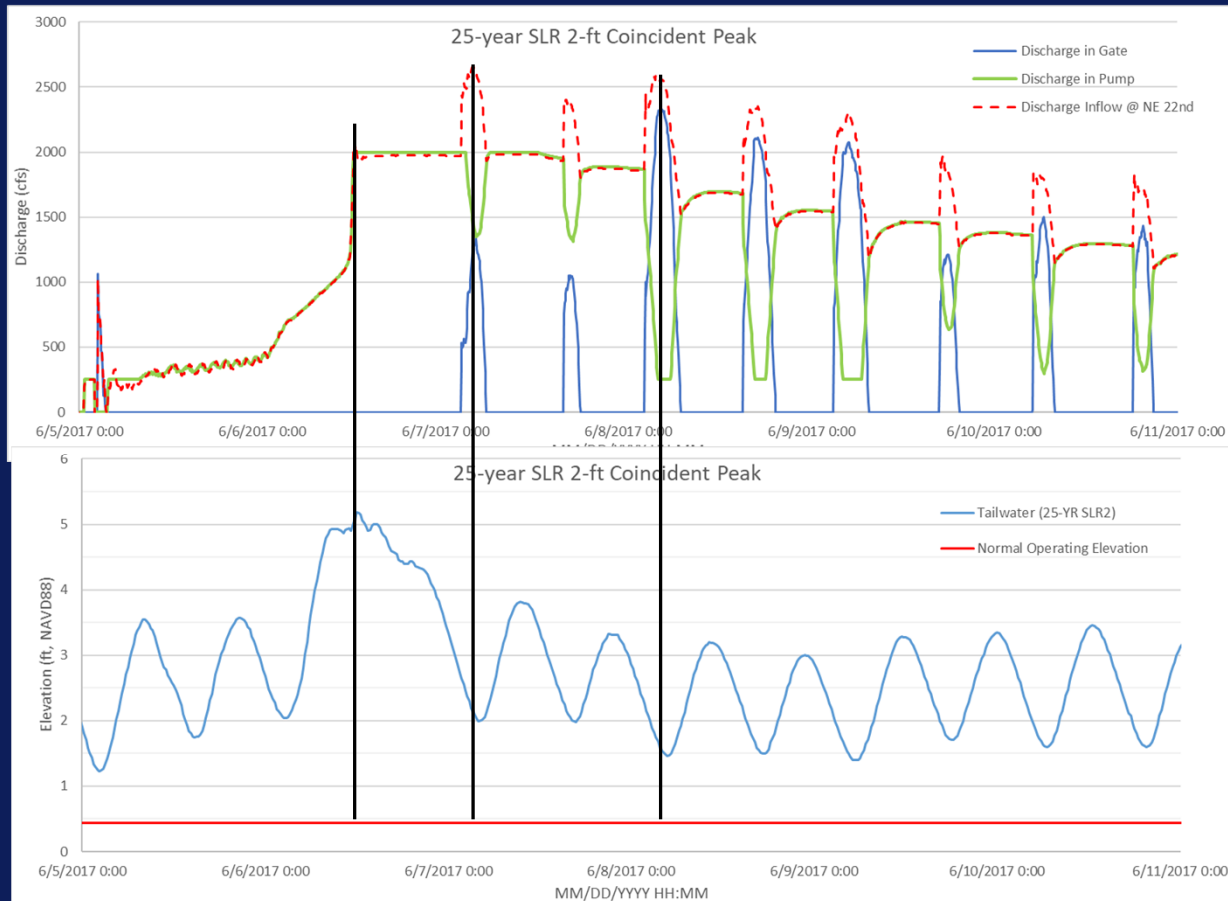


Figure 8.3-7: Instantaneous Discharge and Stage at S-29 Structure for 100-Year Current Conditions Design Storm

Tailwater Boundaries



SRH-2D Boundary Curves



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Model Going Dry – Why?

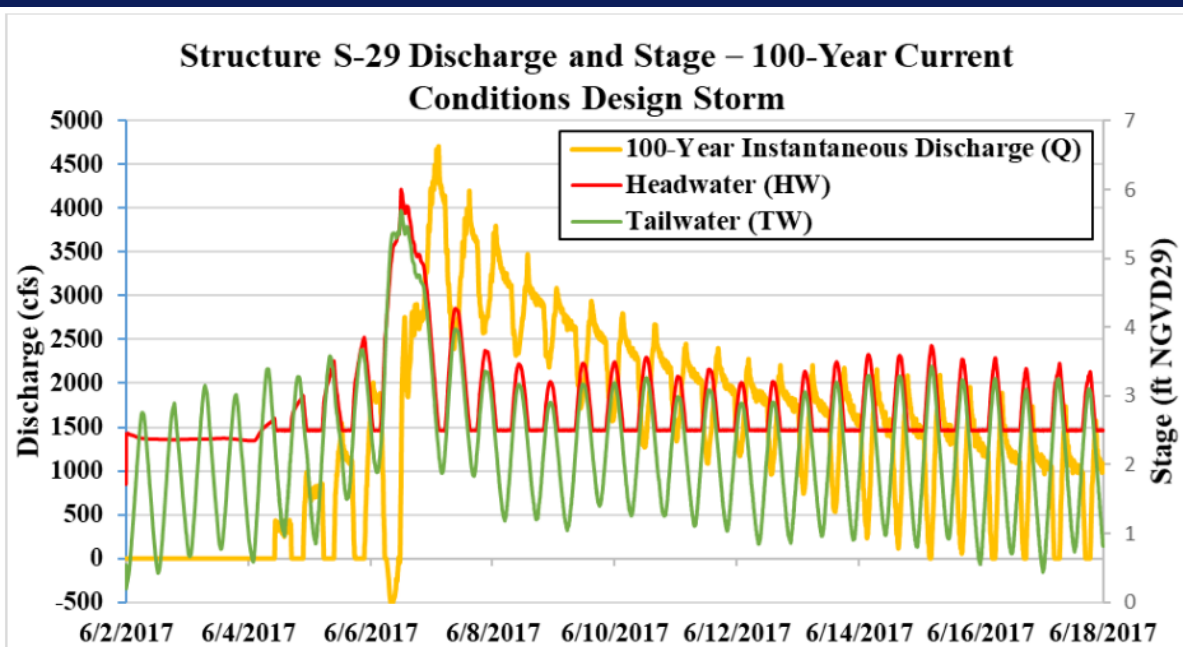
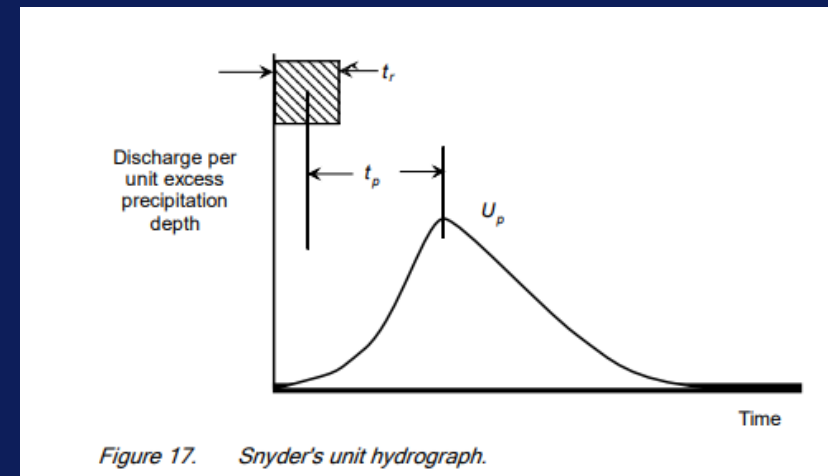


Figure 8.3-7: Instantaneous Discharge and Stage at S-29 Structure for 100-Year Current Conditions Design Storm



Structure S-29 Discharge and Stage – 100-Year Current Conditions Design Storm

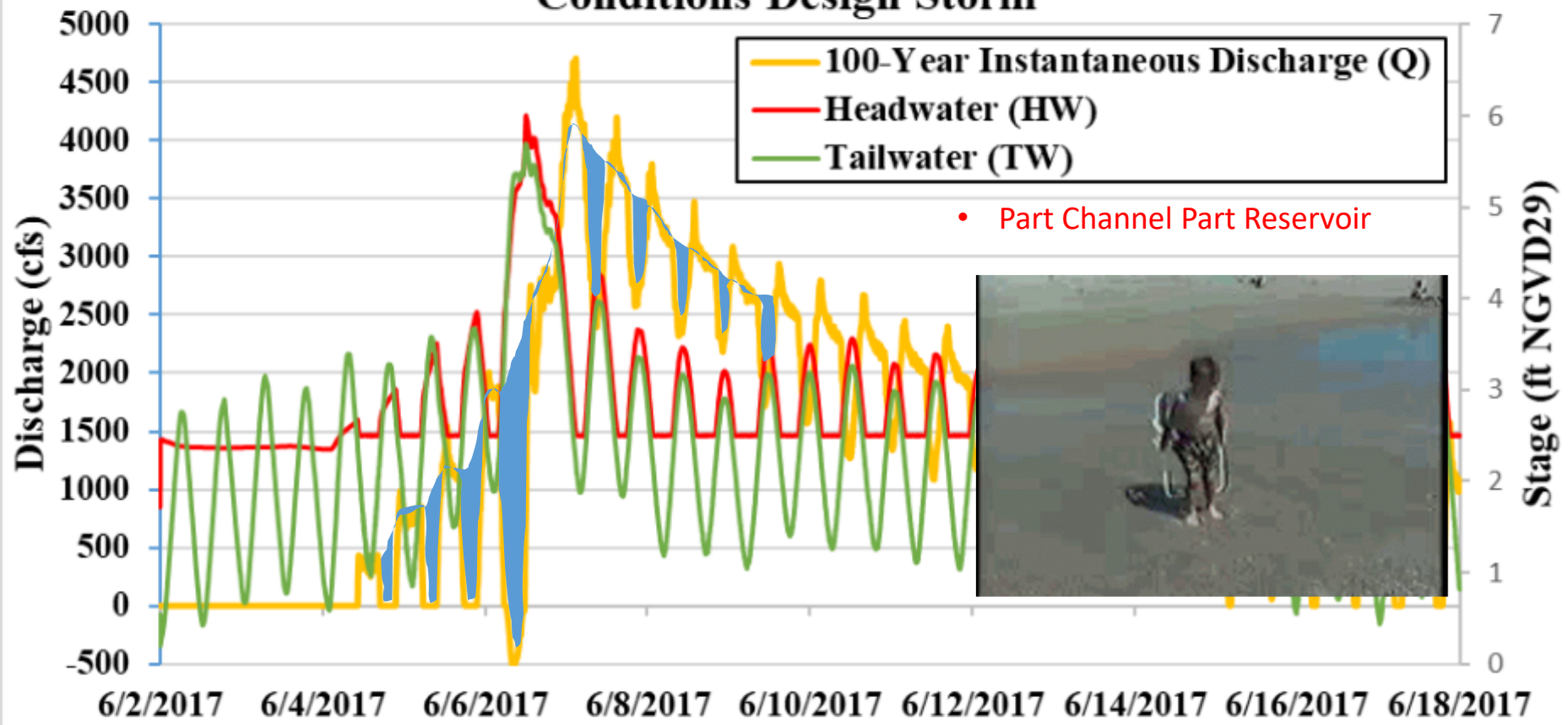


Figure 8.3-7: Instantaneous Discharge and Stage at S-29 Structure for 100-Year Current Conditions Design Storm

Preliminary Results

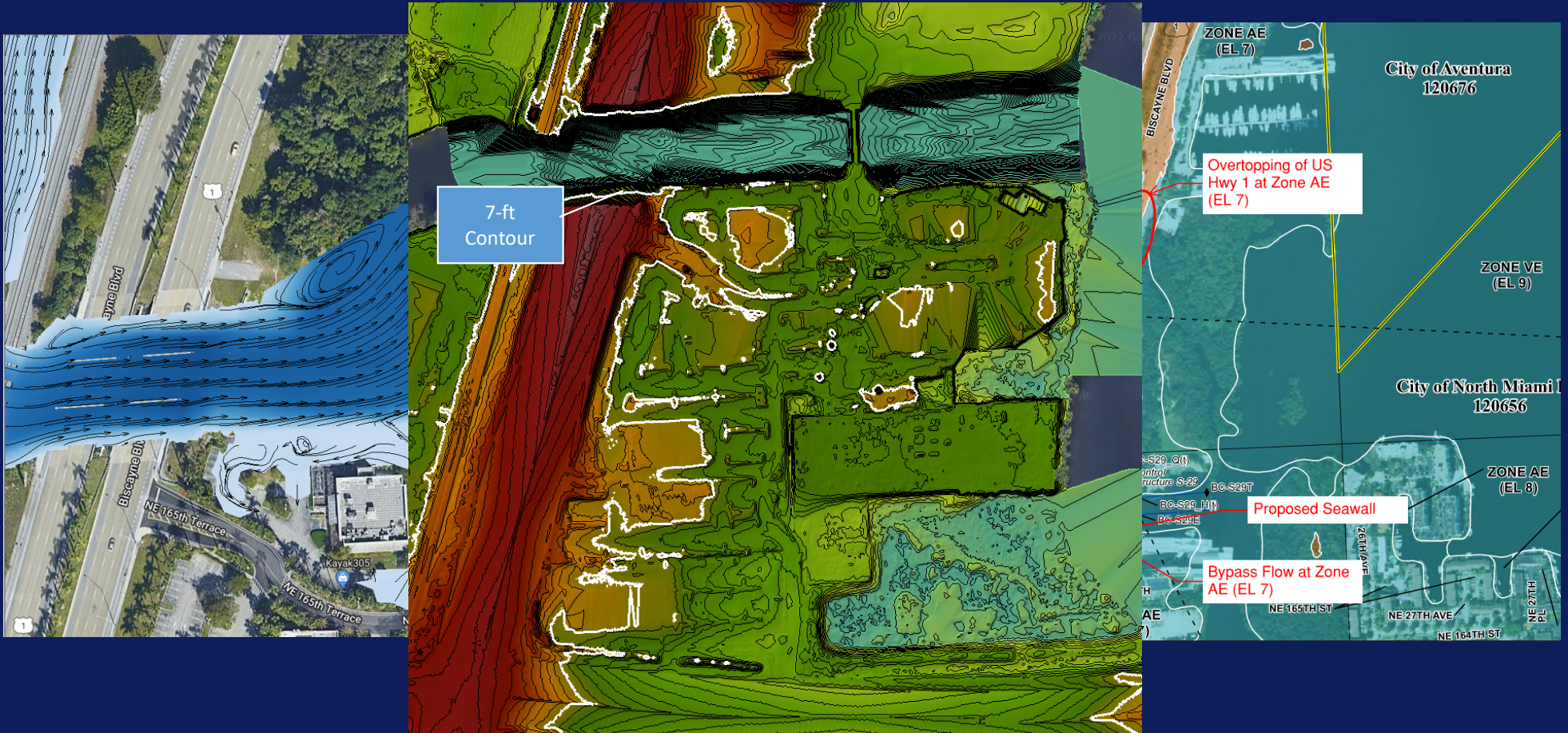
- No Hydrology – Scope Assumption
- Proceed with Steady State
 - Temporarily
- Control WS and Discharge
- Testing 3 Pump Options
 - Flow Patterns and Distribution
 - Low WS
 - High WS
 - Gate Open vs Closed



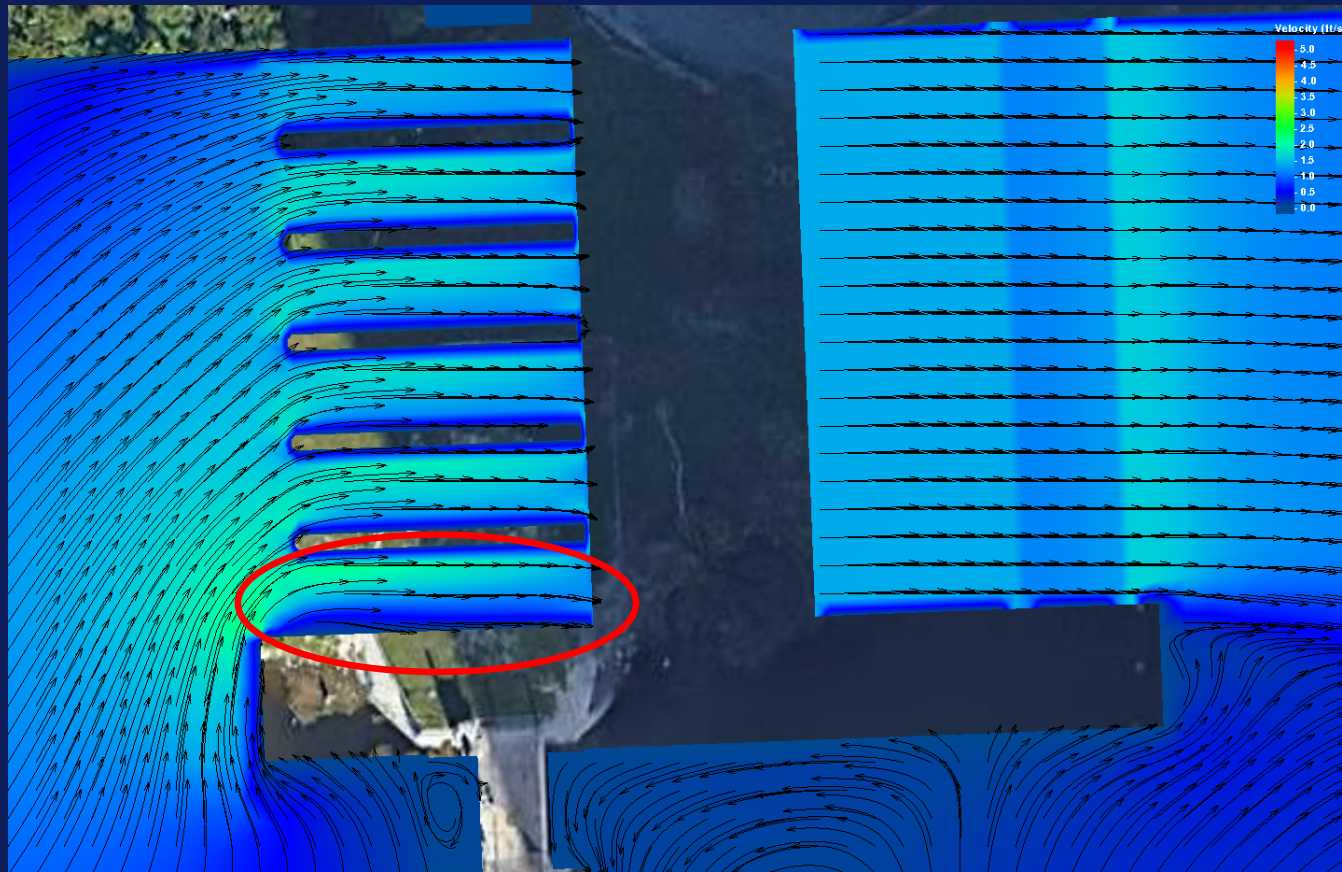
Figure 1 is an aerial photograph of the study area, overlaid with a blue-shaded region representing the proposed development site. The site is bounded by Biscayne Blvd to the west and NE 165th Terrace to the south. A large, dark, irregular shape in the center of the site indicates the proposed building footprint. The surrounding area includes a parking lot, a road intersection, and some existing buildings. A depth scale on the right side of the image ranges from 0.0 to 30.0 feet, with increments of 3.0 feet. The scale is color-coded, with blue representing 0.0 to 3.0 feet, green representing 3.0 to 6.0 feet, yellow representing 6.0 to 9.0 feet, orange representing 9.0 to 12.0 feet, red representing 12.0 to 15.0 feet, and brown representing 15.0 to 30.0 feet. The scale is oriented vertically, with 0.0 at the bottom and 30.0 at the top.



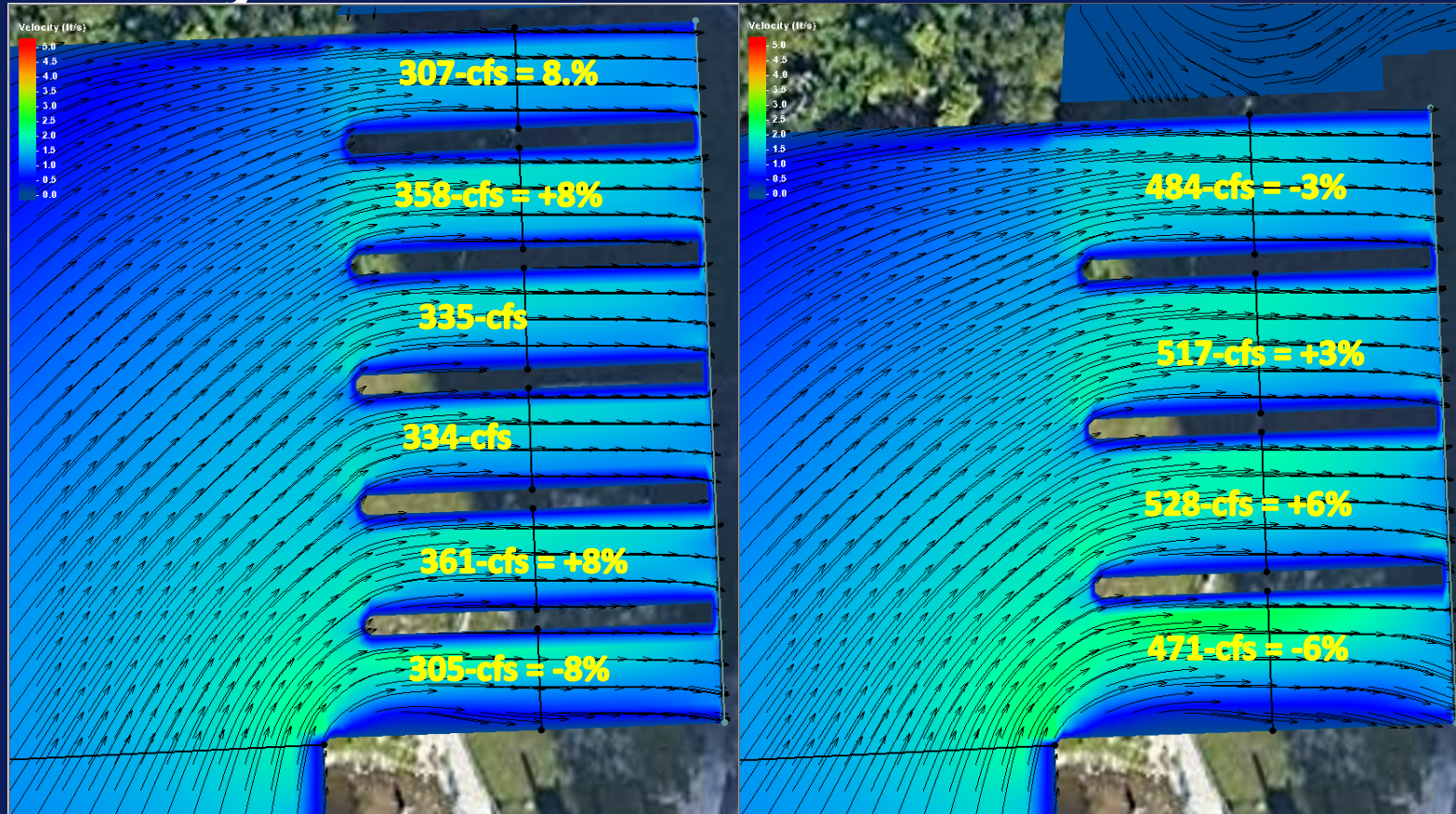
Preliminary Results – Bypass Flow



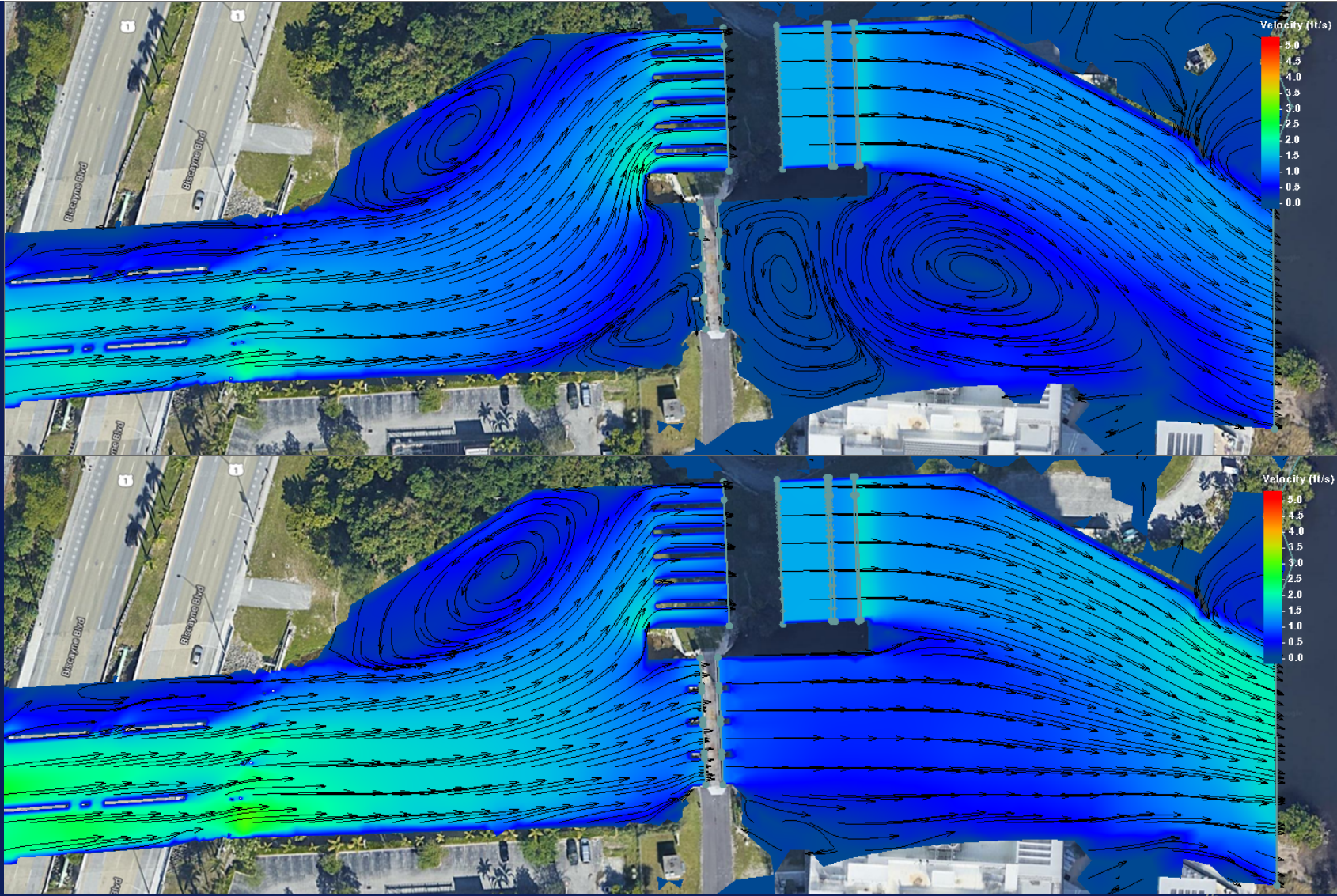
Preliminary Results – Flow Distribution



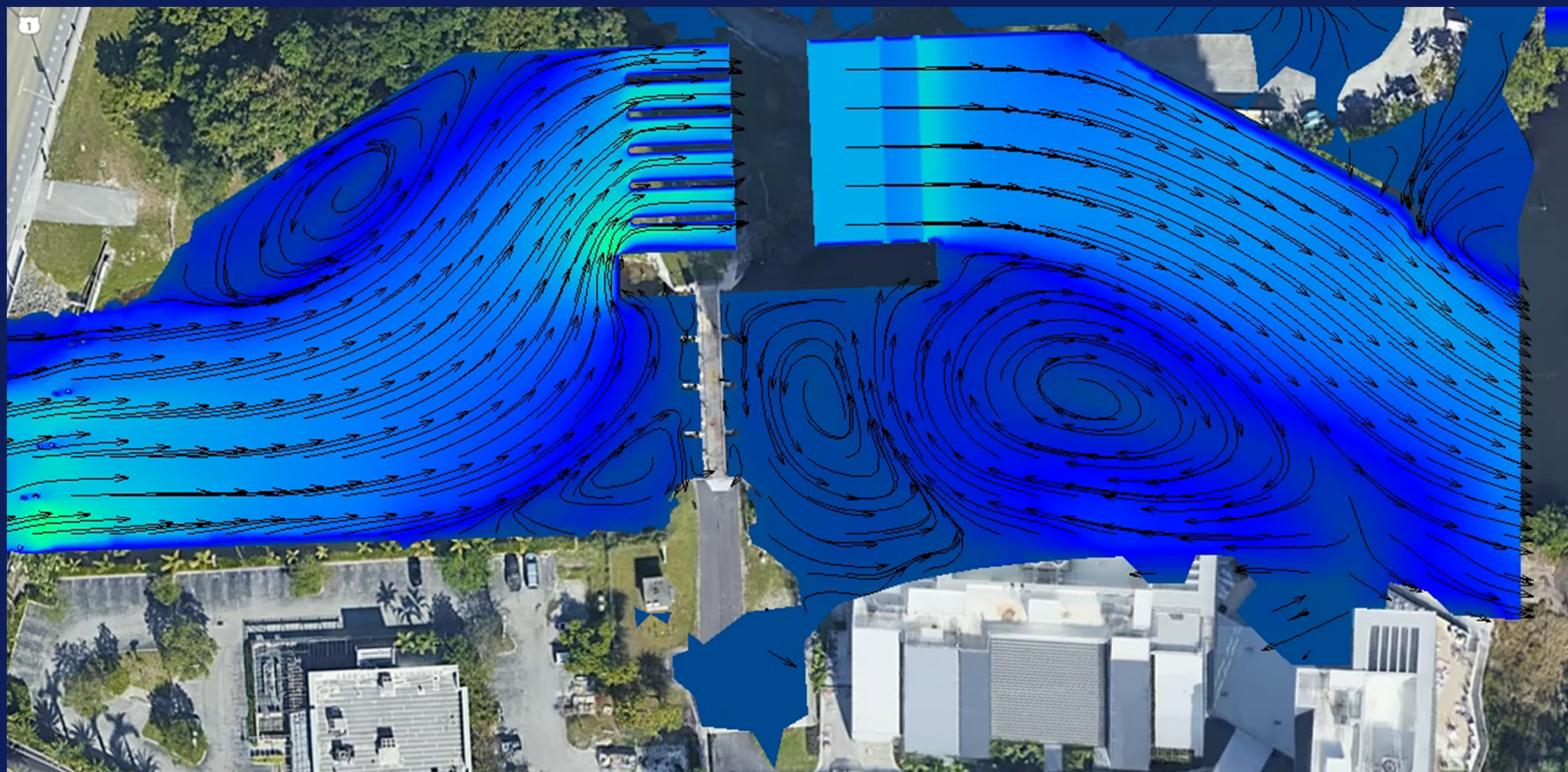
Preliminary Results – Flow Distribution



Gate Open vs Closed

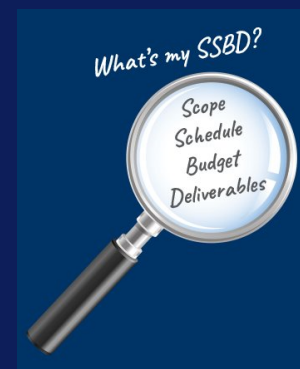


Gate Open vs Closed

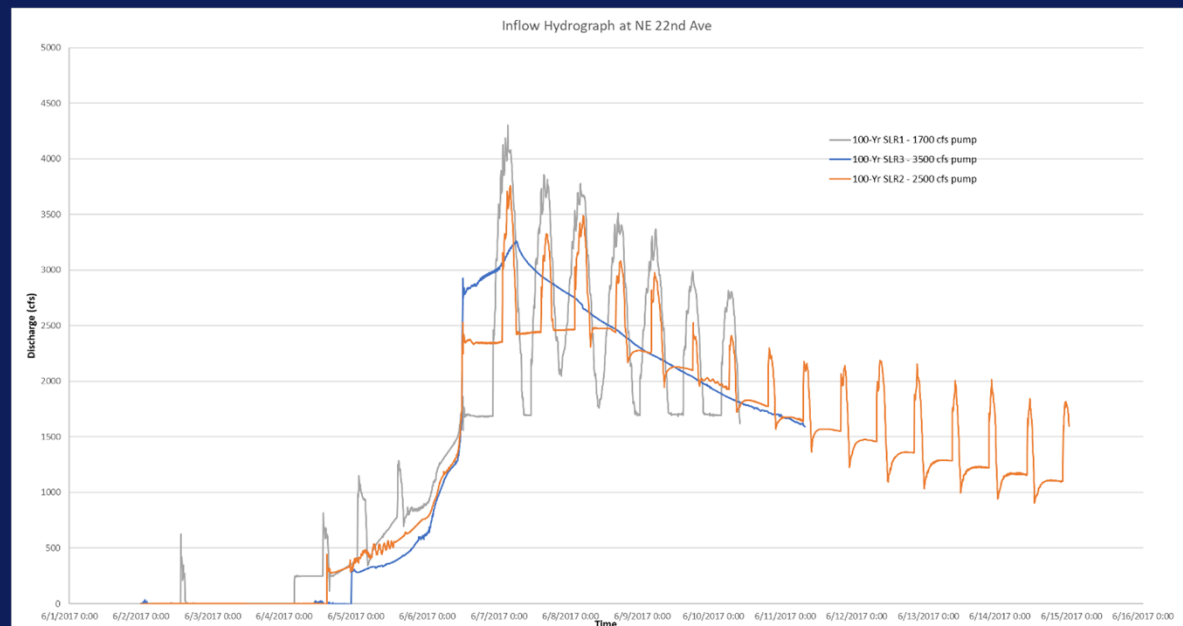




Expanded Scope

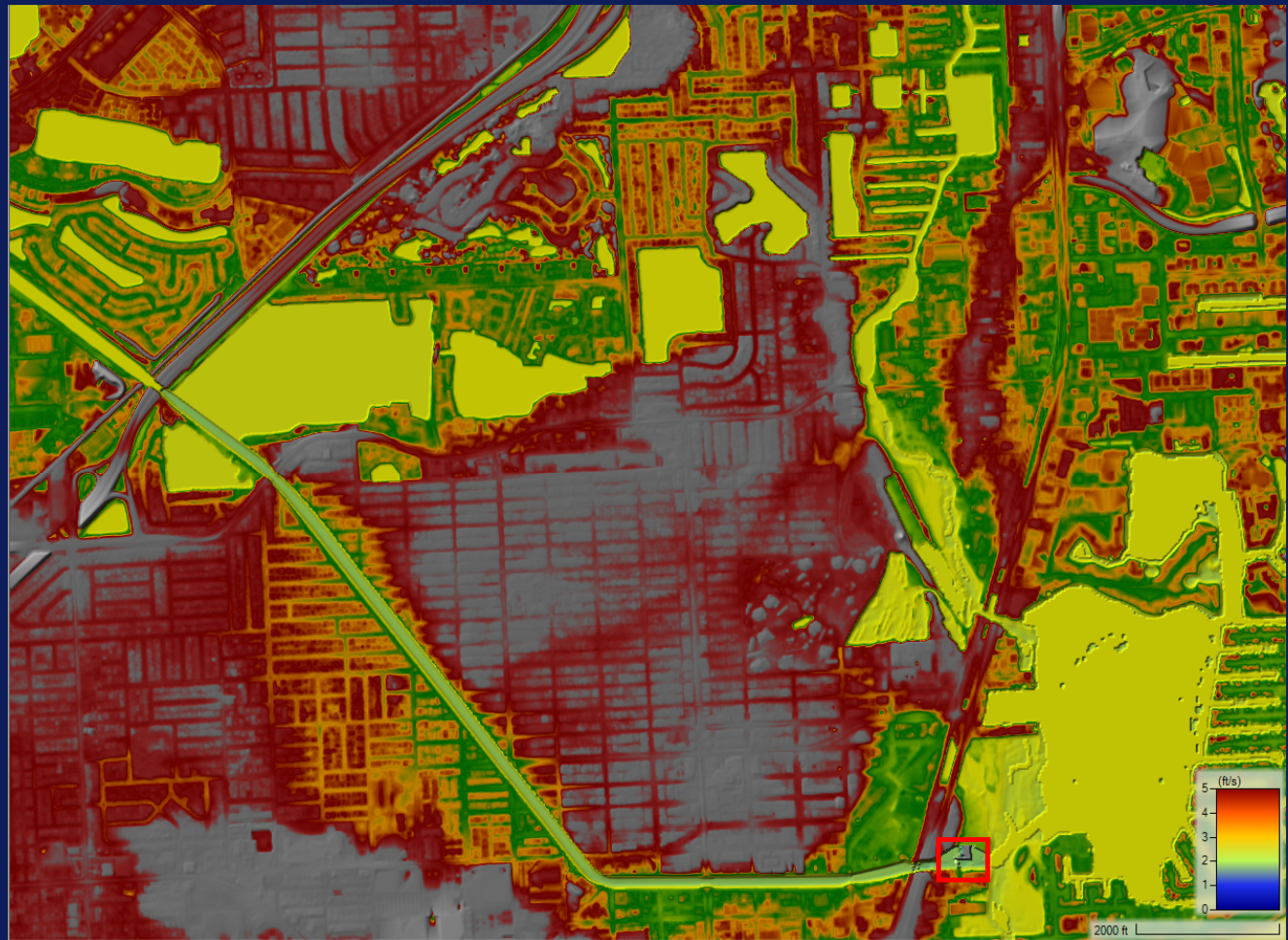


- Determine Pump Type & Sequencing
- Can't rely on FPLOS Study
- Need to Capture Volume
 - Schedules Don't Align



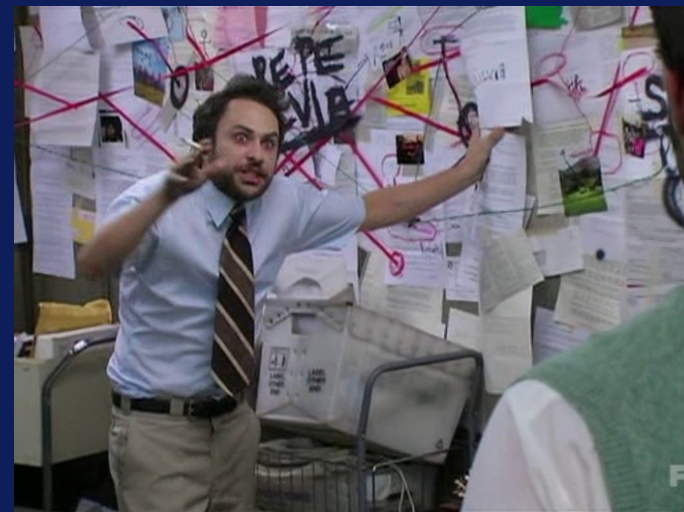
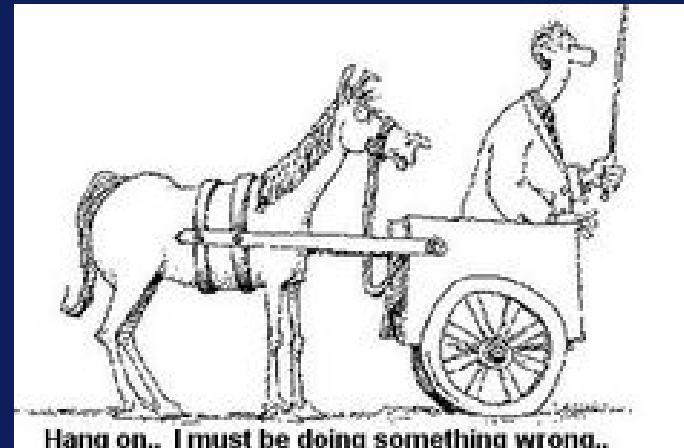
FPLOS Study Limitations

- FPLOS Doesn't Include Storage (Outside of Channel)
 - Tested Adding Upstream Lake
 - Major Impacts
 - Still Missing Floodplain Storage
 - Static Peak Pump rate



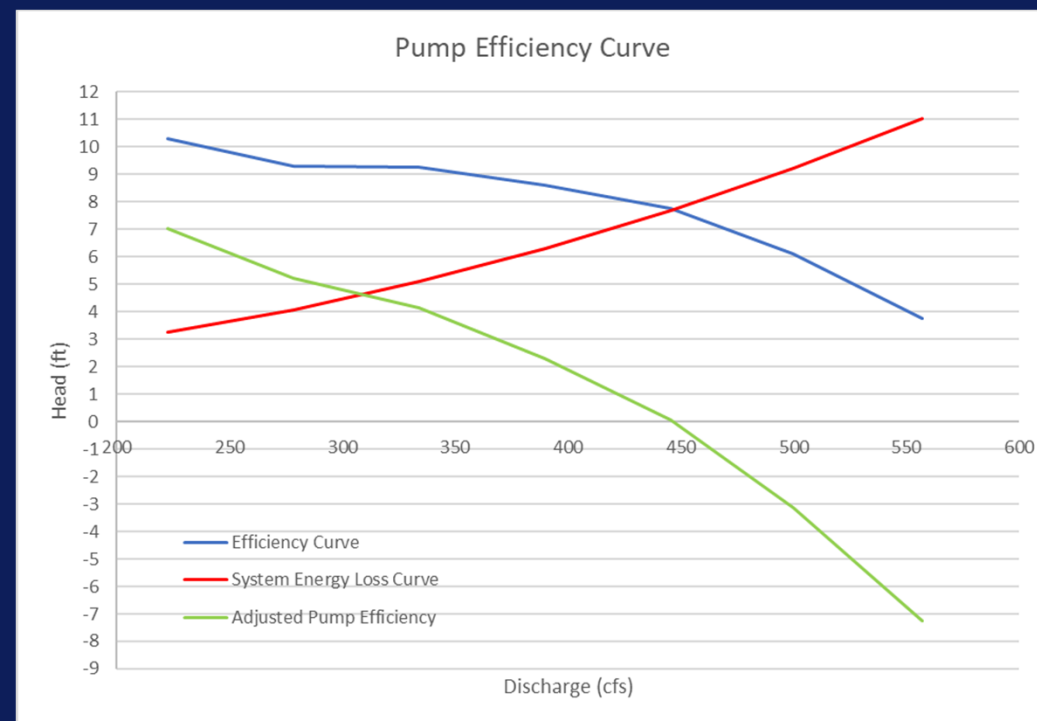
Identify Variables

- Groundwater
 - Influenced by head
- Tide/SLR impacts gates
- Headwater impacts gates
- Different Pumps = Different Efficiency = Variable Discharge
 - Pump selection informed by HW
 - Pump type changes Bay
- On/Off Elevation
- Sequencing
 - 5 pumps turn on independently
- **Changes to WS impact inflow hydrograph!**



Reduce Variables

- 4 flows with 4 SLR
 - 16 Runs per Alternative
- Narrowed to 500-cfs option
- Supplement FPLOS hydrographs
 - HEC-RAS 2D
- Pick 1 Pump type
 - Efficiency doesn't change
 - Still iterate on/off
- New pump station geometry
 - Modeling start over



HEC-RAS 2D

- Larger Domain
 - Capture Storage
 - Reduce HW influence on inflow
- Better Control of Boundaries
 - Pump Boundary
 - Gate Boundary
 - Rules
- Inform SRH-2D Model

Elevation Controlled Gates

SA Conn: Gate

Gate Group: Gate #1

Reference: Based on difference in stage

Stage Difference Reference (First minus Second)

First Reference: 2D Flow Area: S-29

Second Reference: 2D Flow Area: S-29

Stage difference at which gate begins to open: 0.3

Stage difference at which gate begins to close: 0.09144

Gate Opening Rate:(ft/min): 1

Gate Closing Rate:(ft/min): 1

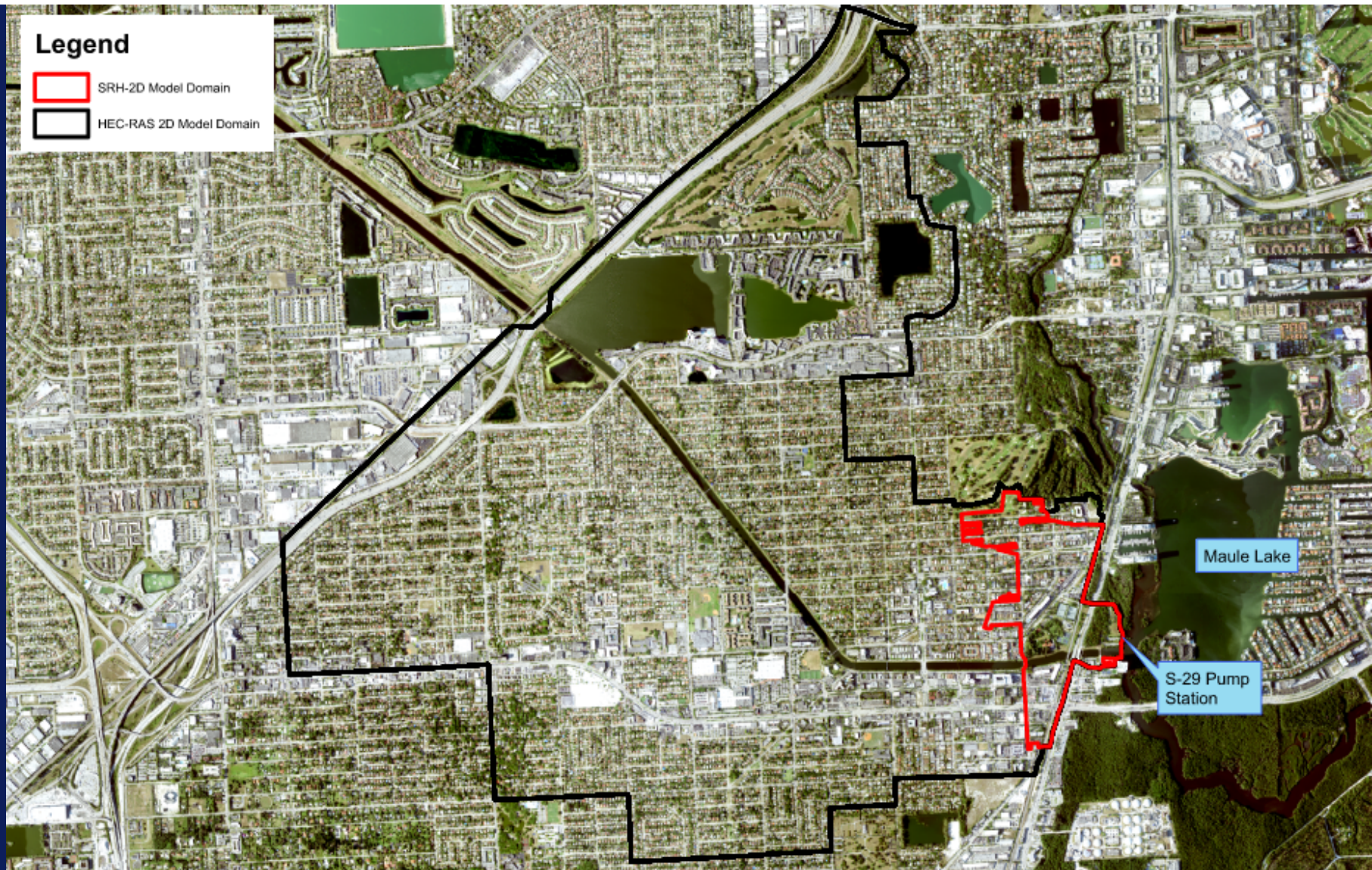
Maximum Gate Opening: 21.76

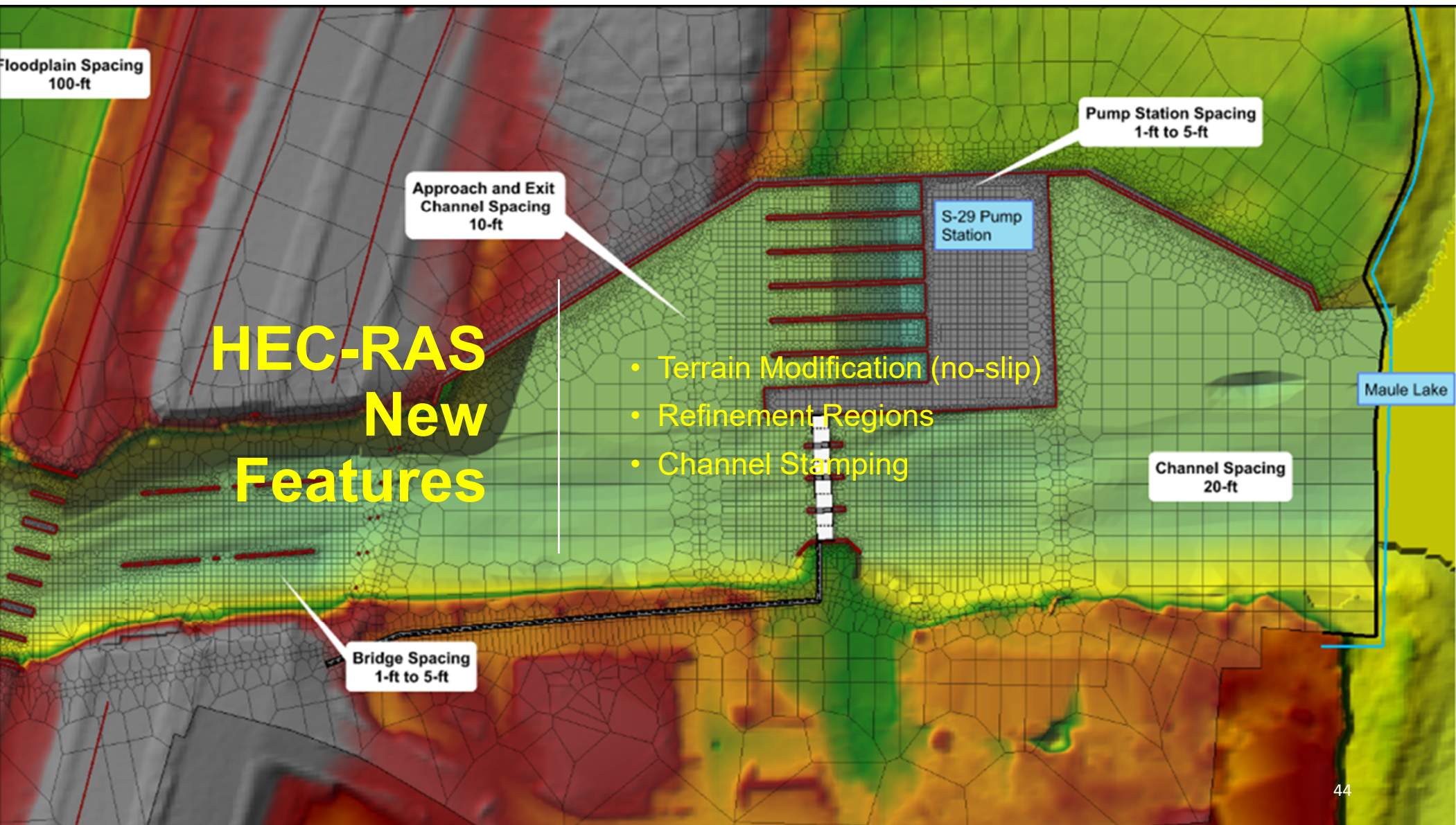
Minimum Gate Opening: 0

Initial Gate Opening (Optional): 0

OK Cancel

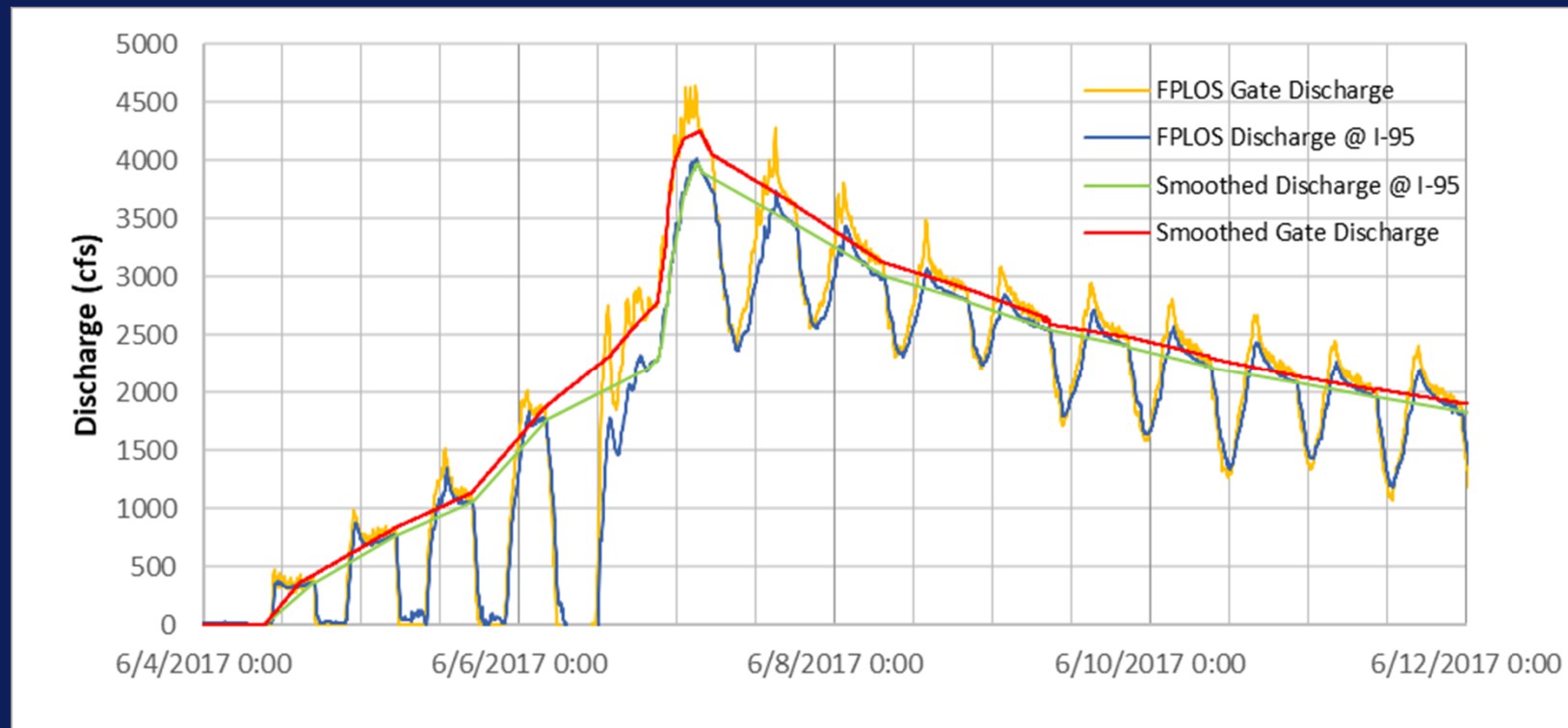
- Capture Volume
- Key to On/Off



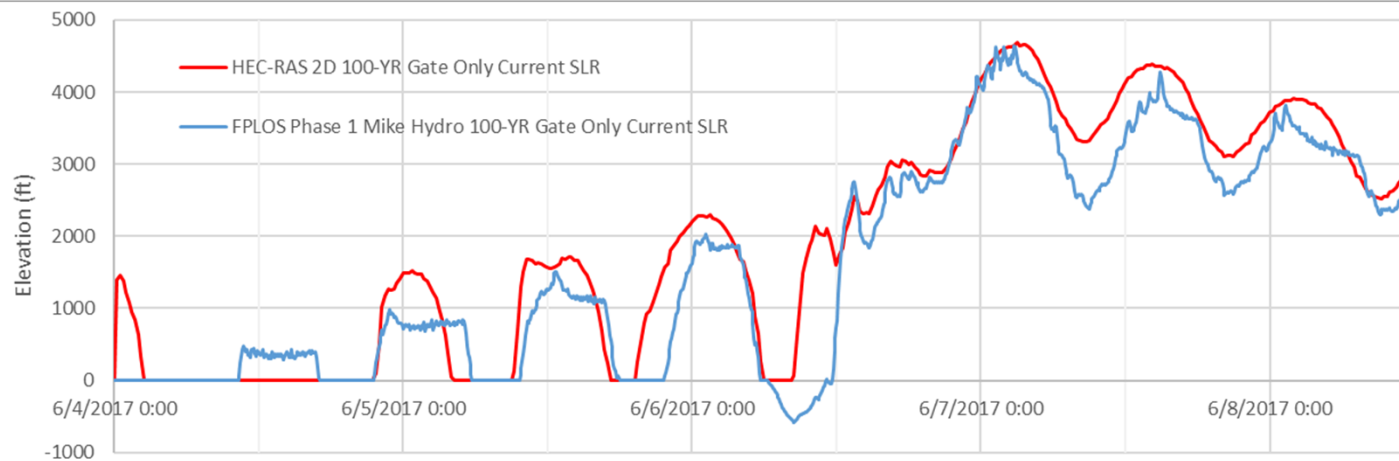
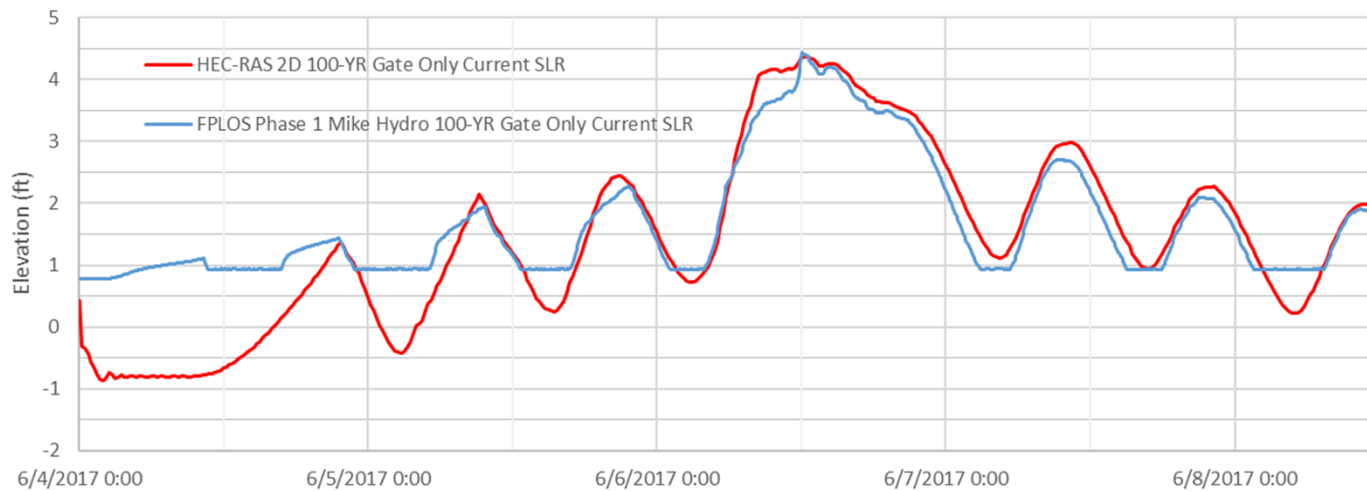


Smoothed Inflow Hydrographs

- Capture Volume
 - Conservative
- FPLOS
 - Ongoing
 - Coordination



Calibration



Running The Model

- 4 flood frequencies, 4 sea level rise conditions = 16 runs
 - 19 total with calibration
- 1-2days per simulation
- 6 computers running models
 - Special thanks to:
 - Kurtis Duemler
 - Matthew Kuechenberg
 - Kush Paliwal
 - Mason Johnson



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On/Off

- Iterative Approach
- Start with 100-year
 - Max SLR
- Need Minimum Flow
 - Can't go dry
- SFWMD Operations
- No Significant Changes
 - Variables Reduced

Normal Range

Normal operational range is from 1.5 to 2.5 feet NGVD29.

During very dry conditions, normal operational range may be raised by 0.2 feet to prevent saltwater intrusion.

Low Range

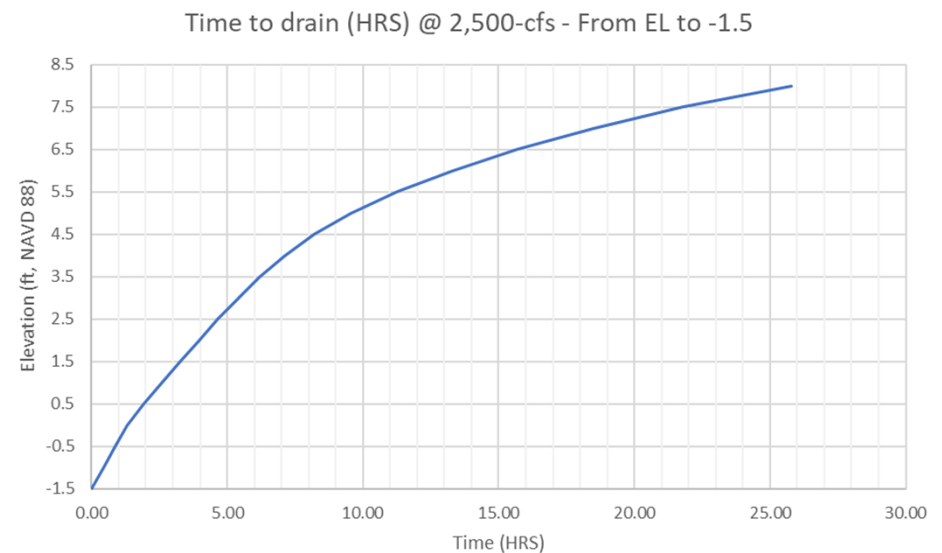
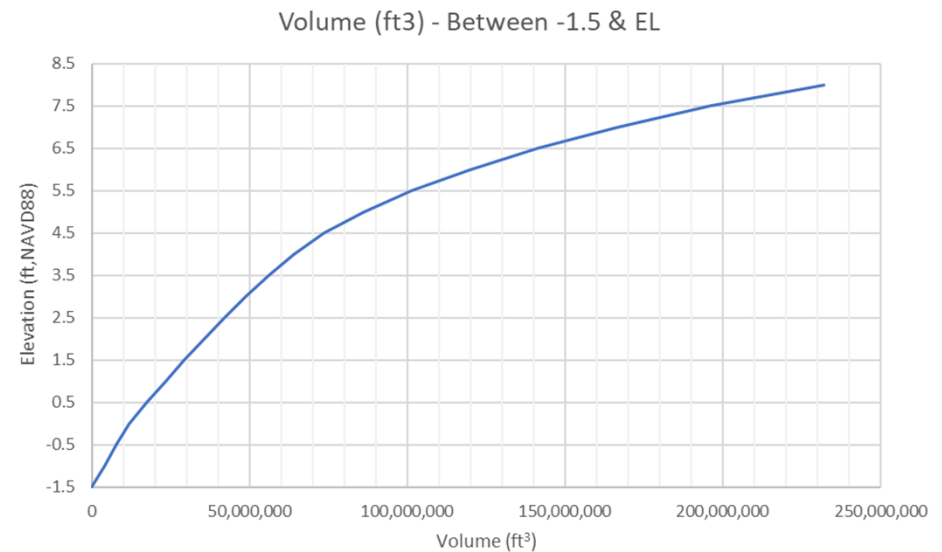
A low range of 1.0 to 1.5 feet NGVD29 is used when additional drainage is required.

During very wet conditions in the C-9 basin an operational range of 0.0 to 1.0 feet NGVD29 may be used.

When the tailwater at S-30 culvert (about 19.0 miles upstream of the S-29 on C-9 Canal) rises above 3.3 feet NGVD29, S-29 is placed on low range until the tailwater at S-30 recedes to 2.7 feet NGVD29 at which time normal operation is resumed at S-29.

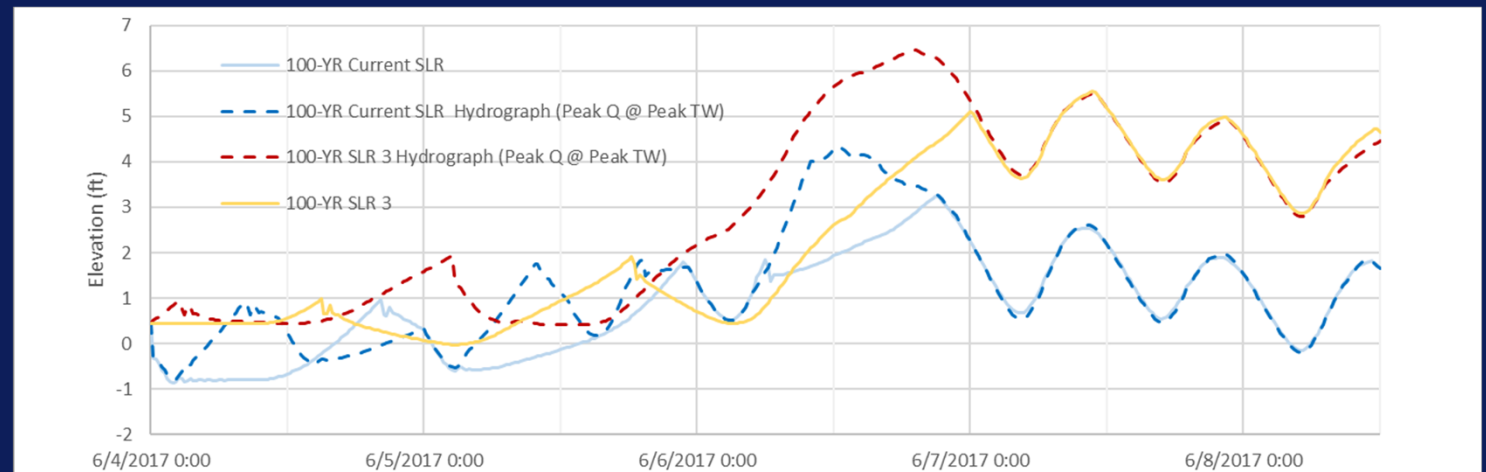
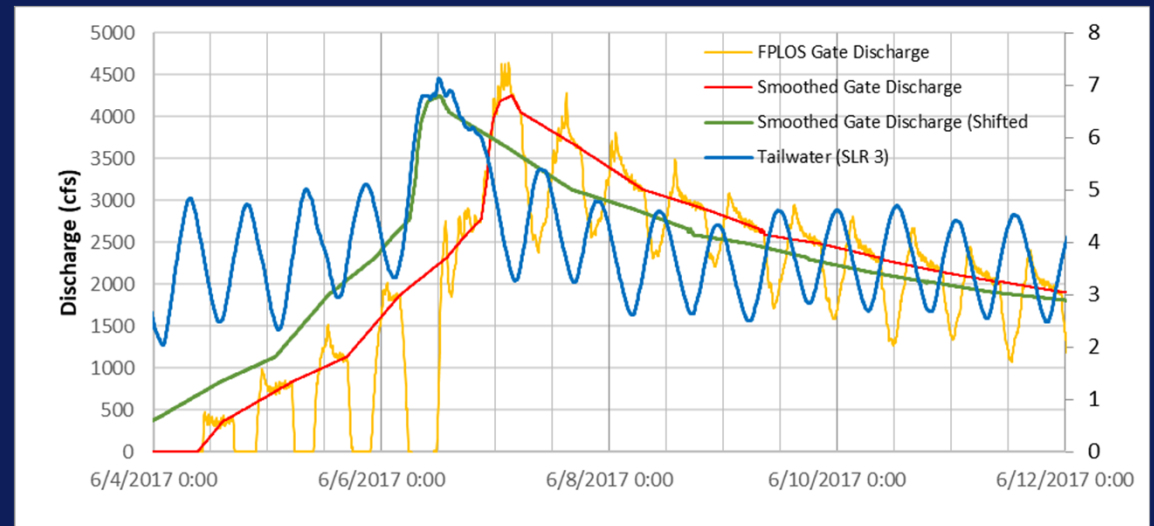
HEC-RAS Results – On/Off Elevations

Elevation (EL)	Volume (ft ³) - Between -1.5 & EL		Time to drain (HRS) @ 2,500-cfs	
	Total	Incremental	Incremental	to EL -1.5
-1.5	0	0	0.00	0.00
-1	3,863,494	3,863,494	0.43	0.43
-0.5	7,743,280	3,879,785	0.43	0.86
0	11,673,982	3,930,702	0.44	1.30
0.5	17,242,499	5,568,517	0.62	1.92
1	23,234,120	5,991,621	0.67	2.58
1.5	29,331,772	6,097,652	0.68	3.26
2	35,572,428	6,240,656	0.69	3.95
2.5	42,013,675	6,441,247	0.72	4.67
3	48,763,142	6,749,467	0.75	5.42
3.5	56,024,536	7,261,394	0.81	6.22
4	64,087,823	8,063,287	0.90	7.12
4.5	73,738,291	9,650,468	1.07	8.19
5	86,059,444	12,321,153	1.37	9.56
5.5	101,347,391	15,287,947	1.70	11.26
6	119,740,484	18,393,093	2.04	13.30
6.5	141,378,863	21,638,379	2.40	15.71
7	166,501,124	25,122,261	2.79	18.50
7.5	196,110,369	29,609,246	3.29	21.79
8	231,993,347	35,882,978	3.99	25.78



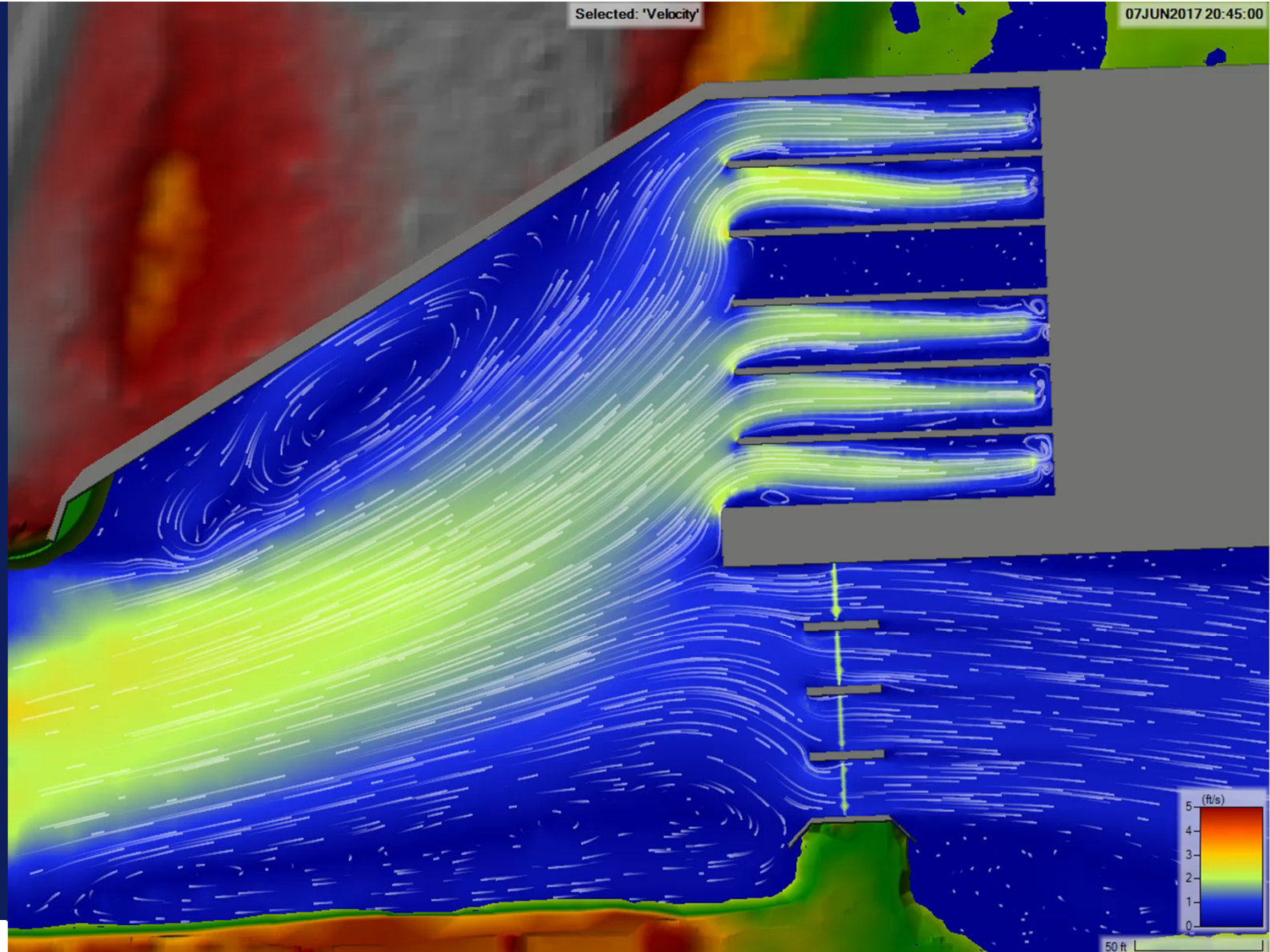
Sensitive to Tailwater

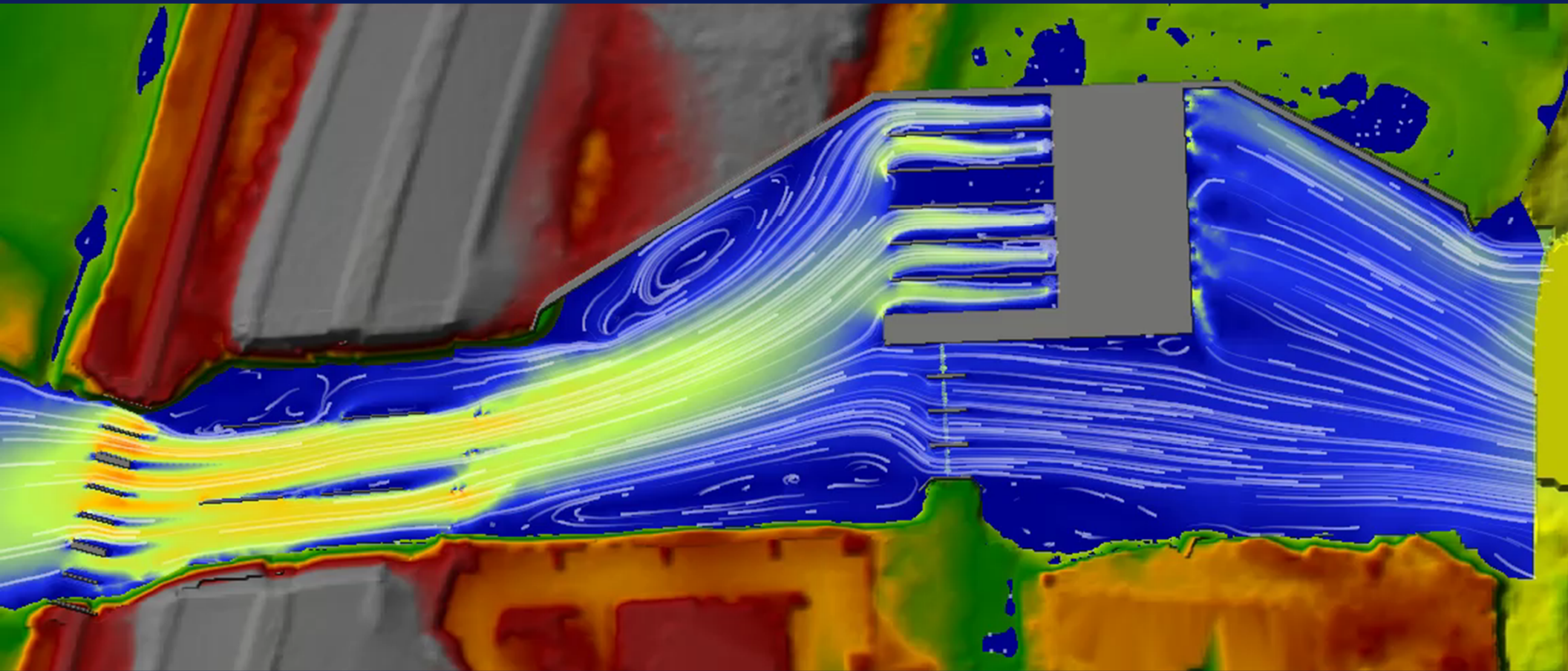
- FPLOS
 - Peak Rainfall at Max TW
- Peak Q at Max TW?



Selected: 'Velocity'

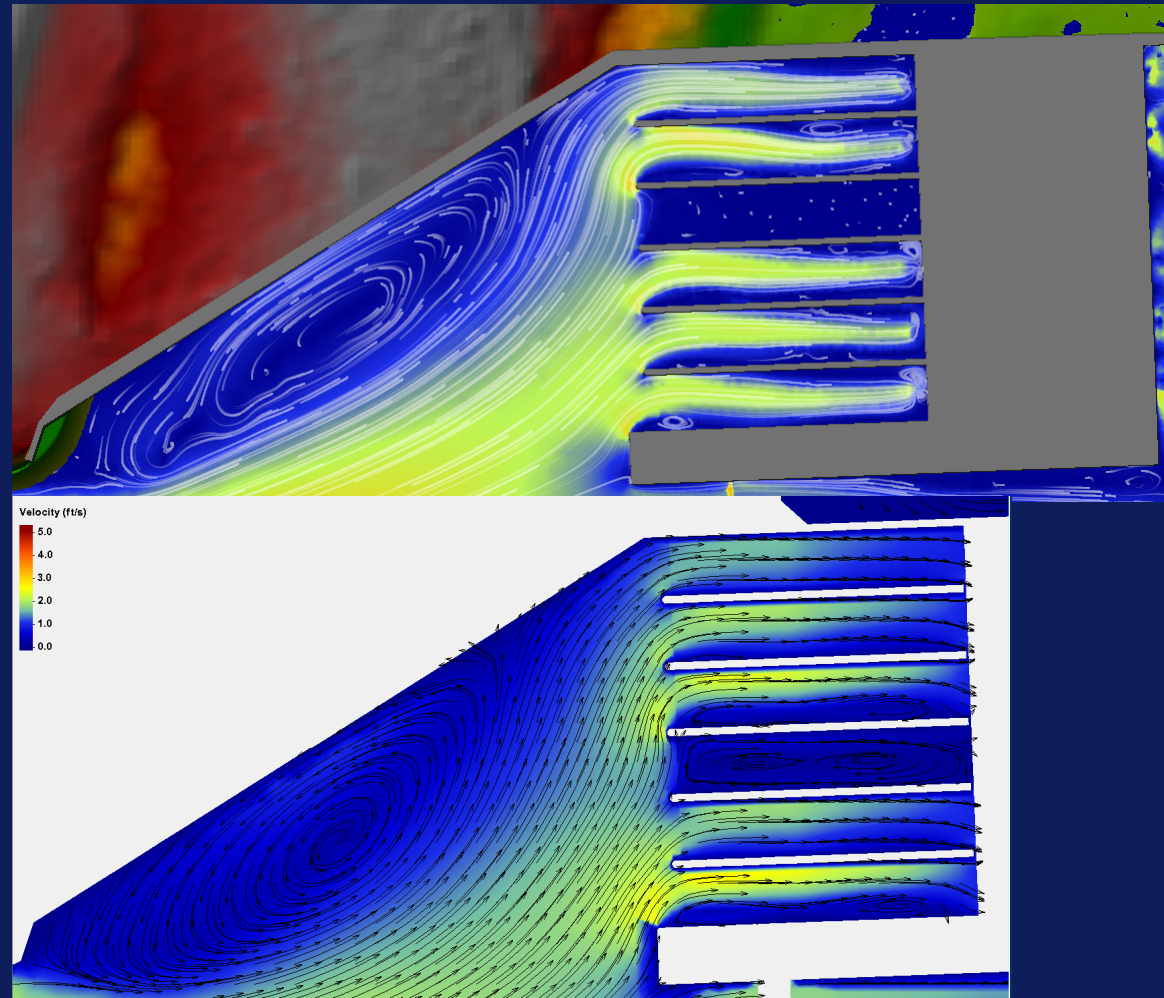
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Back to SRH-2D

- Original Scope
- Results from HEC-RAS
 - Inform Boundaries
- Full St. Venant, Finite Volume, High Density
- No WS comparison



SRH-2D Results – Gate Operation

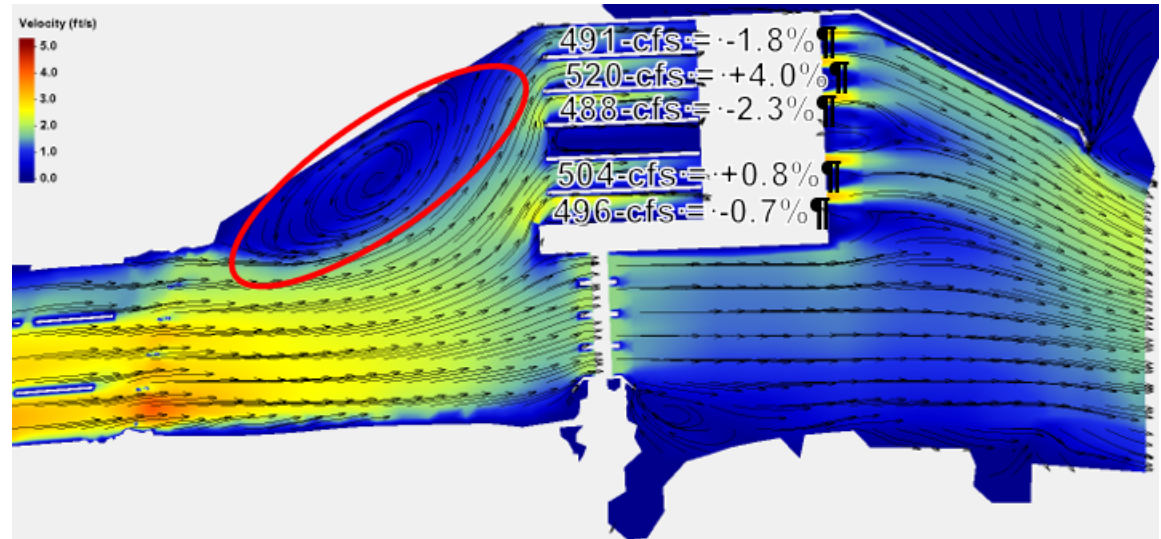


Figure 22: 100-year Maximum Discharge

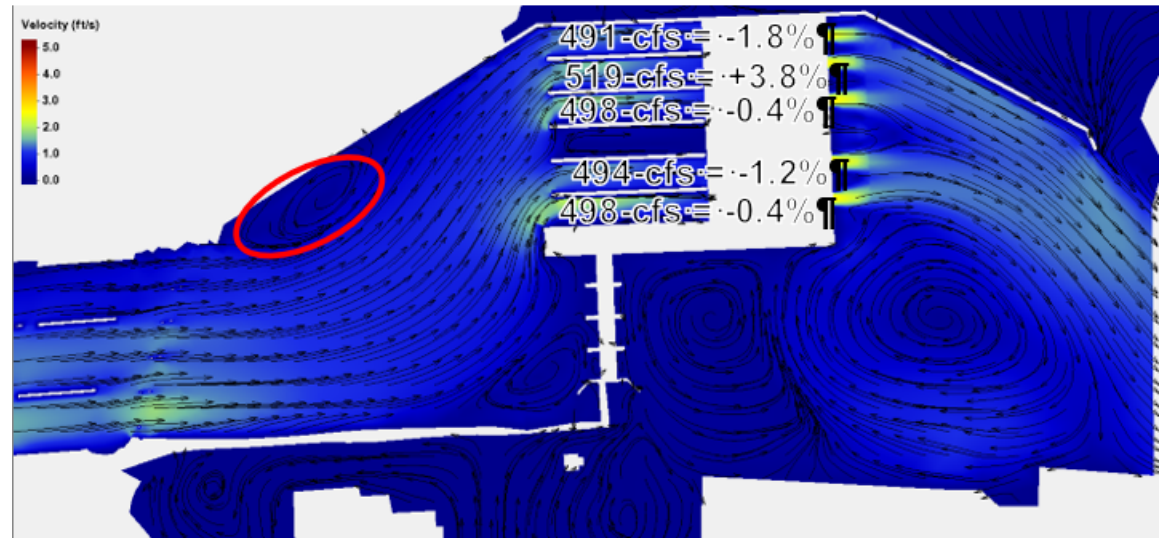


Figure 23: 100-year Maximum Headwater

SRH-2D Results – Headwater

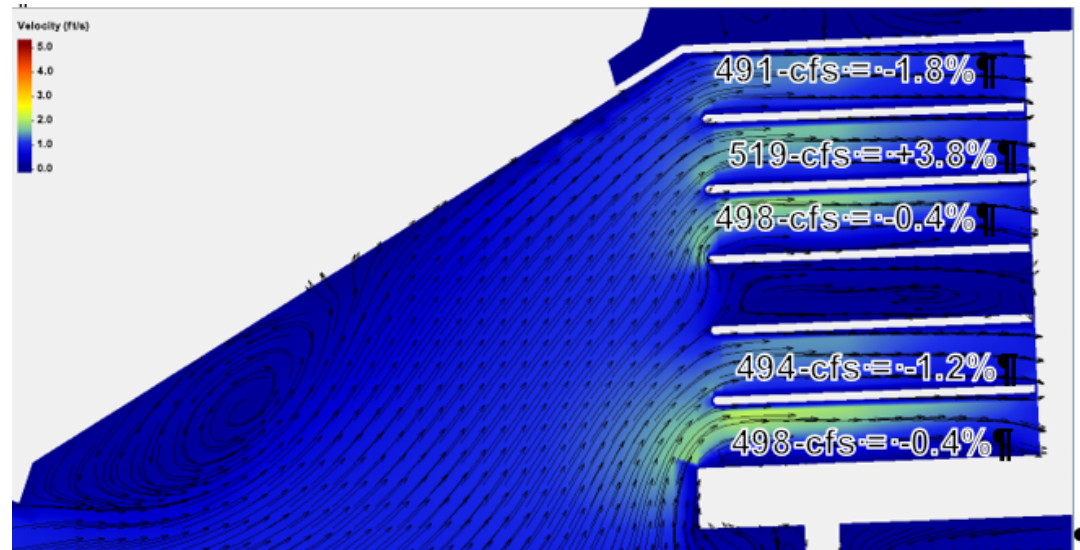


Figure 26: Flow patterns for 100-year maximum headwater

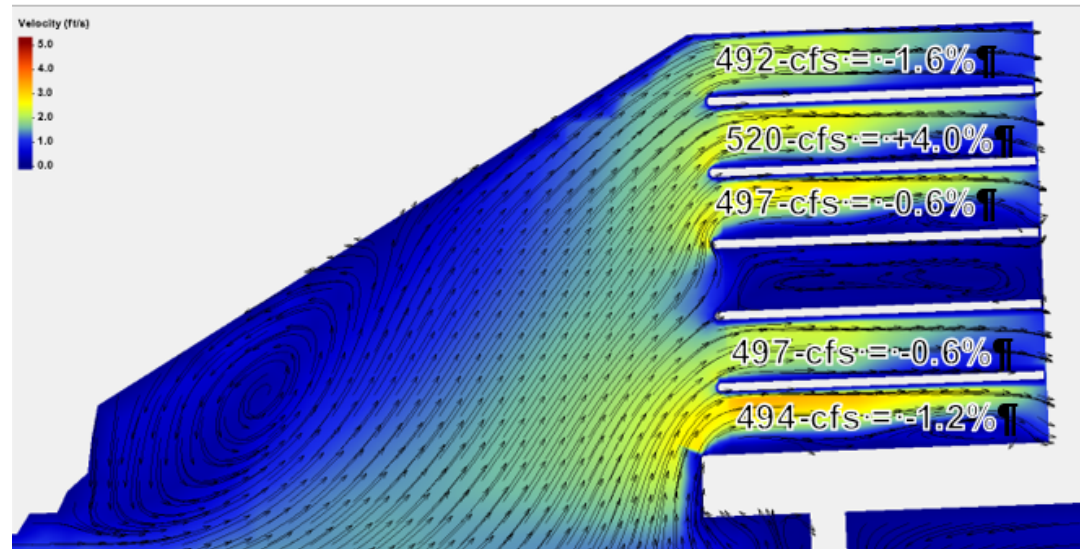


Figure 27: Flow patterns for 100-year minimum headwater

SRH-2D Results – Pump Bays

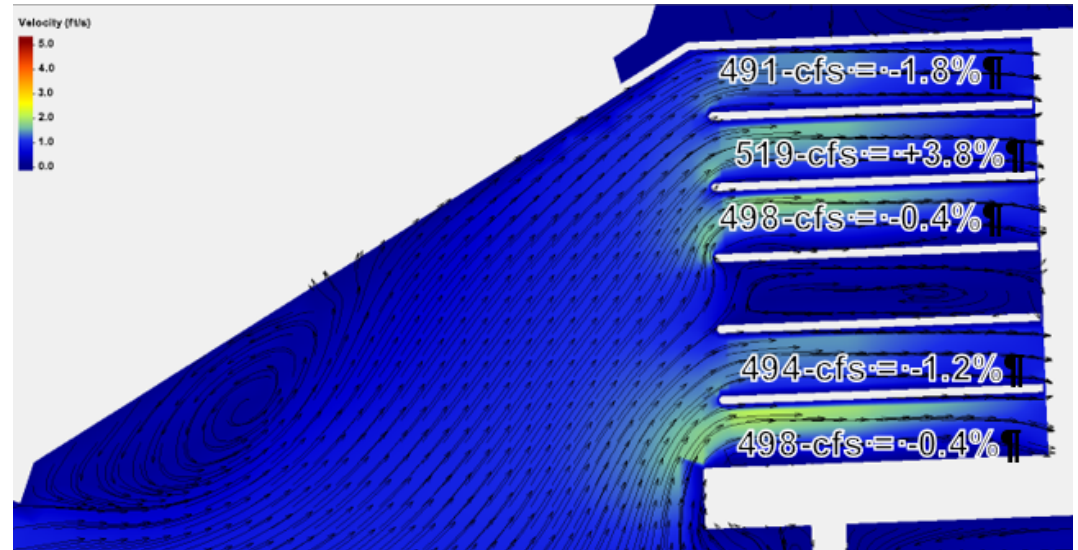


Figure 30: 100-year maximum headwater (Bays 1, 2, 3, 5, and 6)

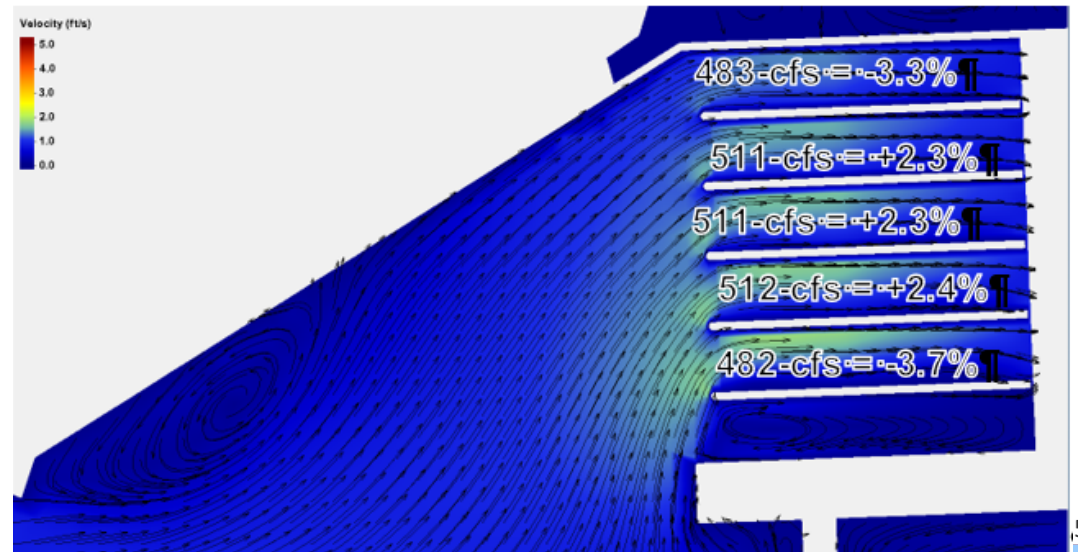


Figure 31: 100-year maximum headwater (Bays 1, 2, 3, 4, and 5)

SRH-2D Results – Pump Bays

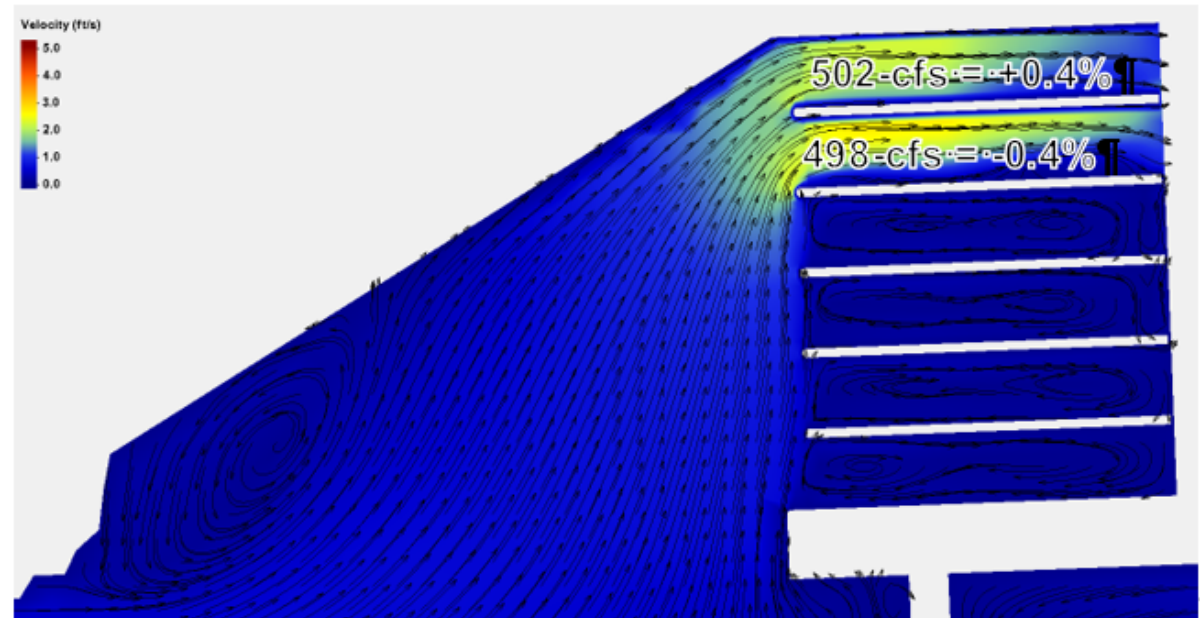
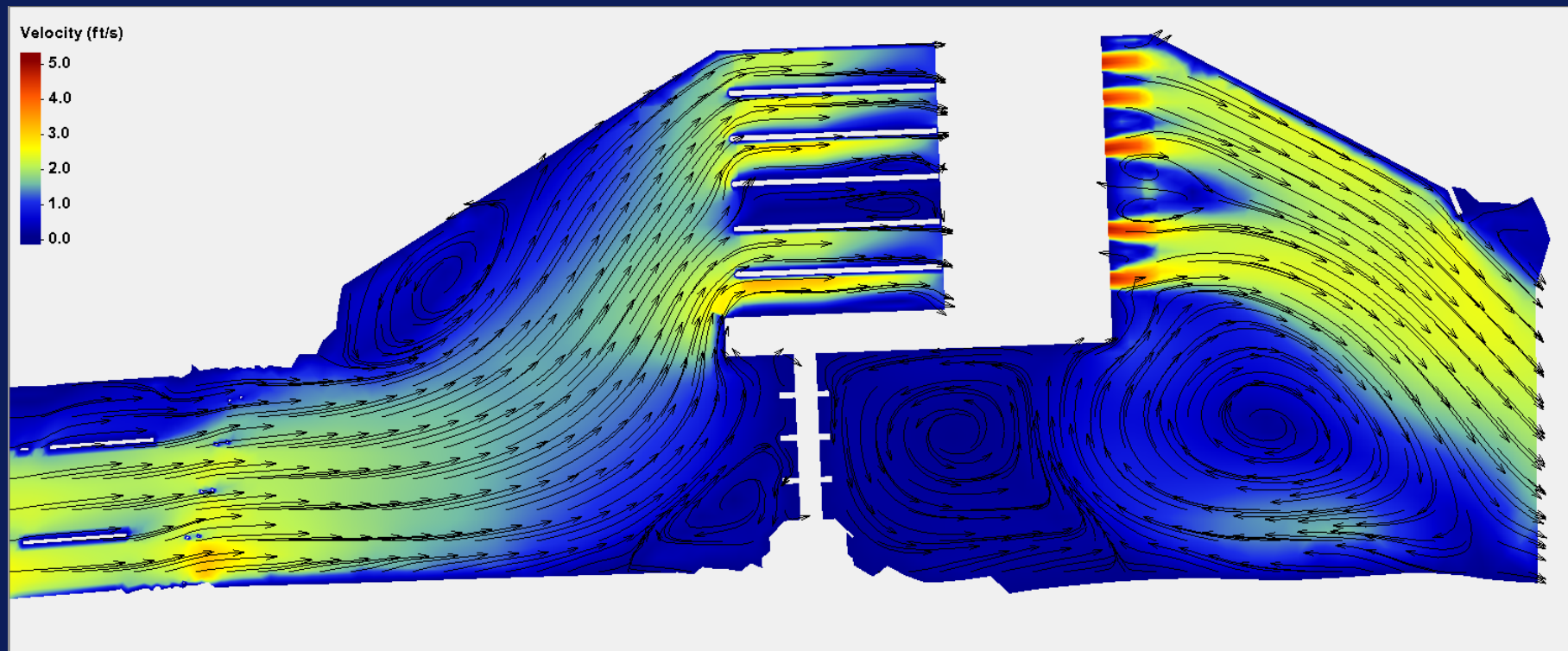


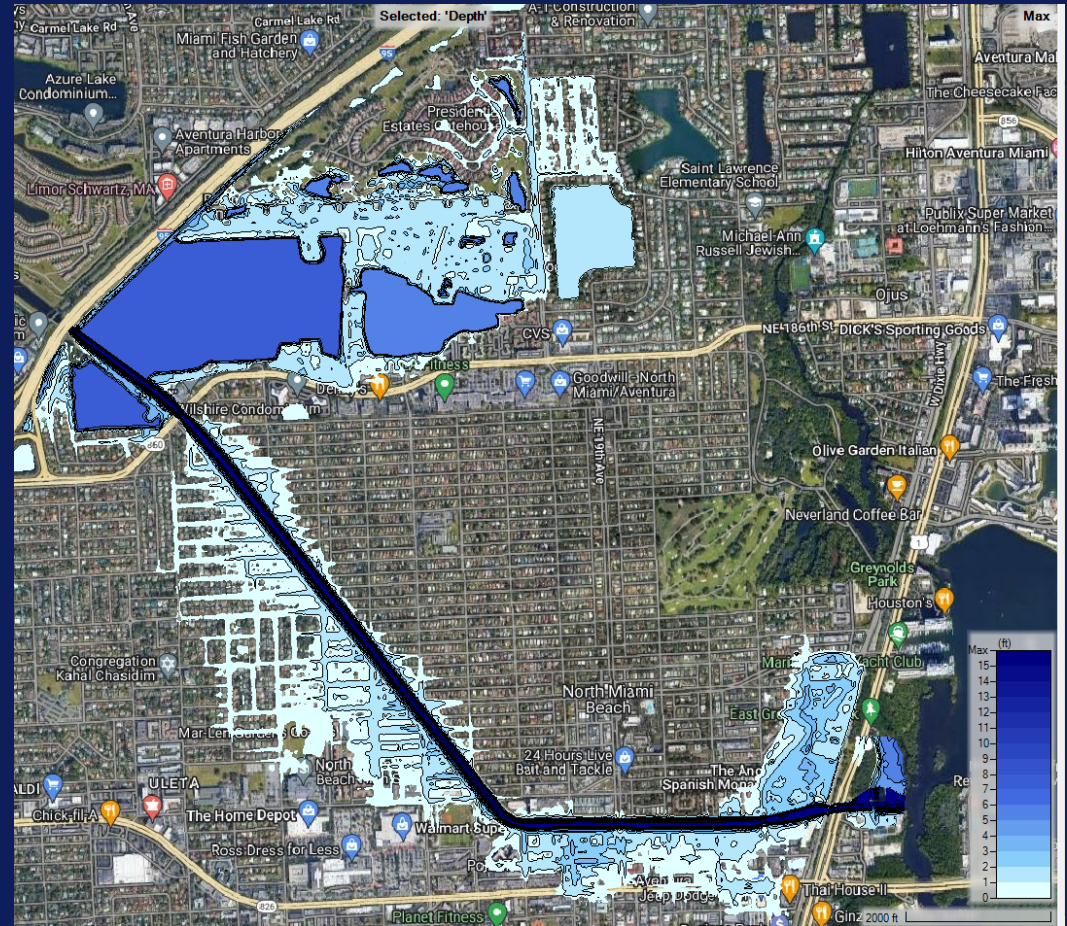
Figure 33: 100-year maximum headwater (Bays 1 and 2)

Low Headwater - Velocity



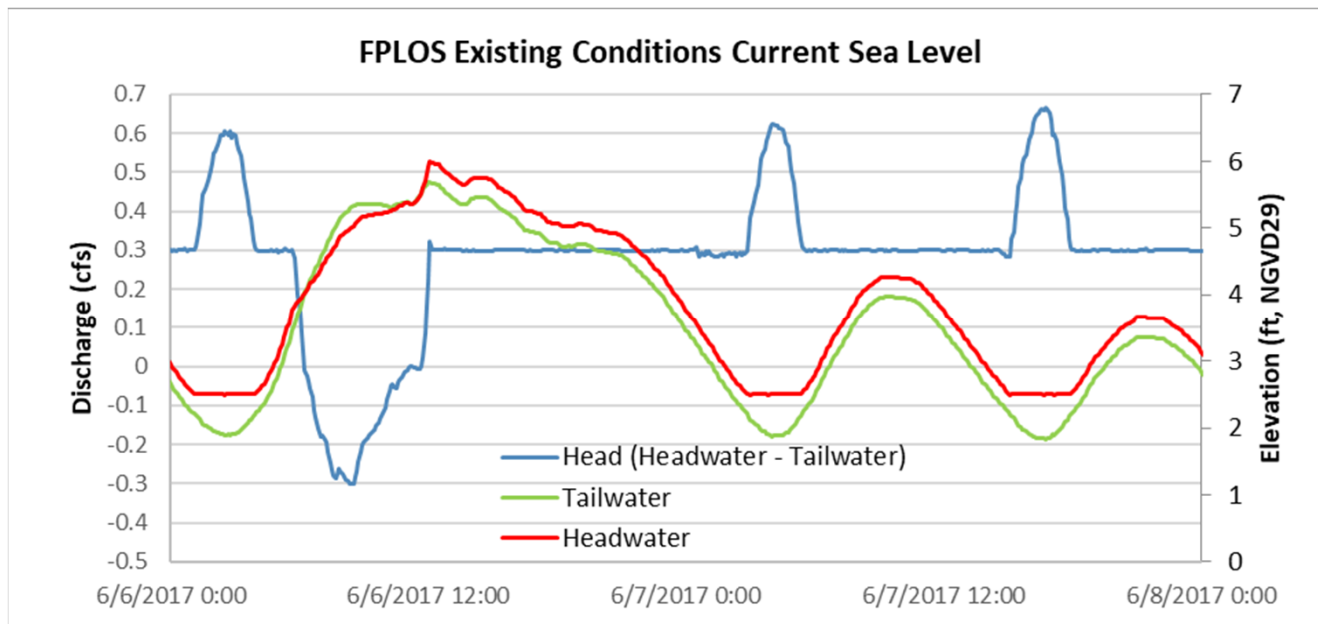
Results and Observations

- Risk of Bypass Flow
- Flow Concentrates Against Bay Wall
- Low Upstream Shear
- High Outlet Velocity
- Simultaneous Gate and Pump
- Pump On/Off
- Coincident Peak
- TW WSEL Control



Results and Observations

Gates Equalize Quickly



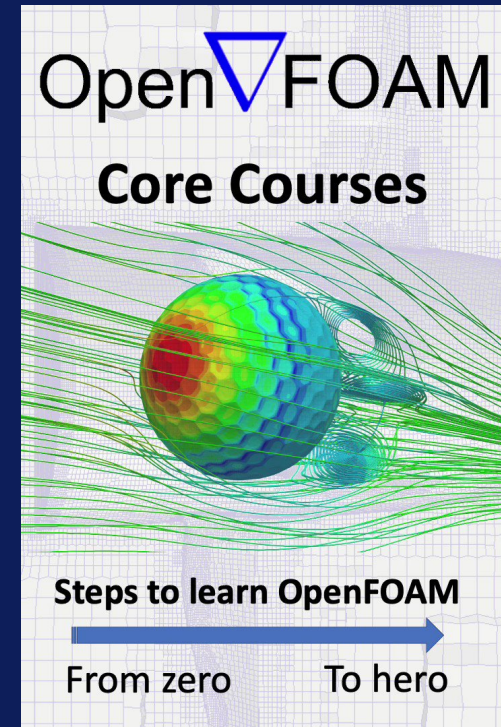
Questions?



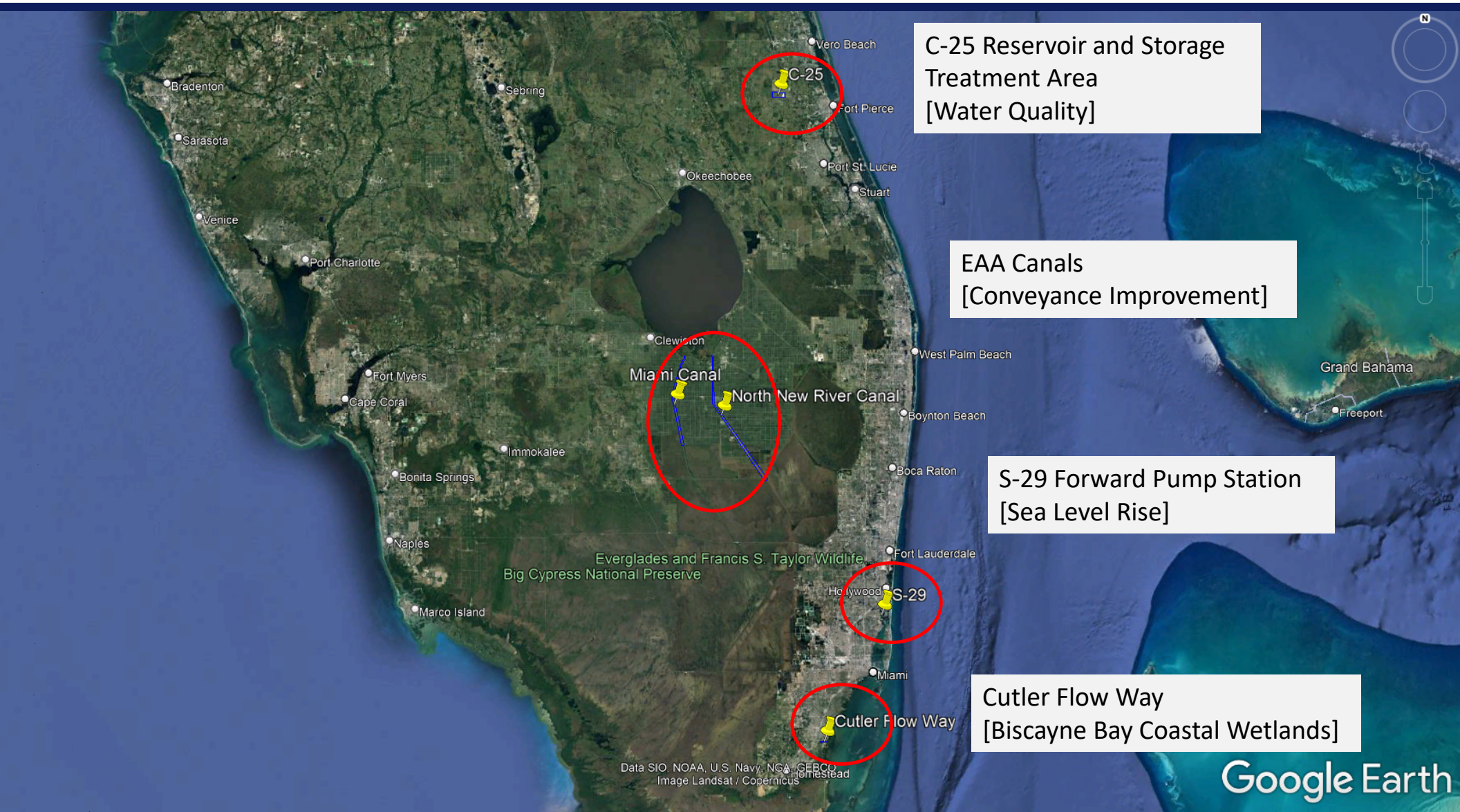
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3D & Physical Modeling



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C-25 Reservoir and Storage
Treatment Area
[Water Quality]

EAA Canals
[Conveyance Improvement]

S-29 Forward Pump Station
[Sea Level Rise]

Cutler Flow Way
[Biscayne Bay Coastal Wetlands]

Data SIO, NOAA, U.S. Navy, NGA, GEBCO
Image Landsat / Copernicus

Google Earth

Planning for System Enhancements

Our Resiliency Vision

Risk Reduction / Effectiveness

Implementation Resources

Anticipated Future Conditions

Vulnerable Population and Critical Infrastructure

Leveraging Partnerships and Public Engagement

Ongoing Ecosystem Restoration Efforts

Innovative Green/Nature-Based Solutions

Offsetting New Energy Demands with Sustainable Sources

SEA LEVEL RISE AND FLOOD RESILIENCY PLAN



DRAFT JUNE 2022



Project Background

- Sea Level Rise
 - 3-ft by 2100
- Annual Flood Losses
 - \$25 Billion by 2050
- SFWMD
 - Forward Pump Stations
 - Pilot Projects

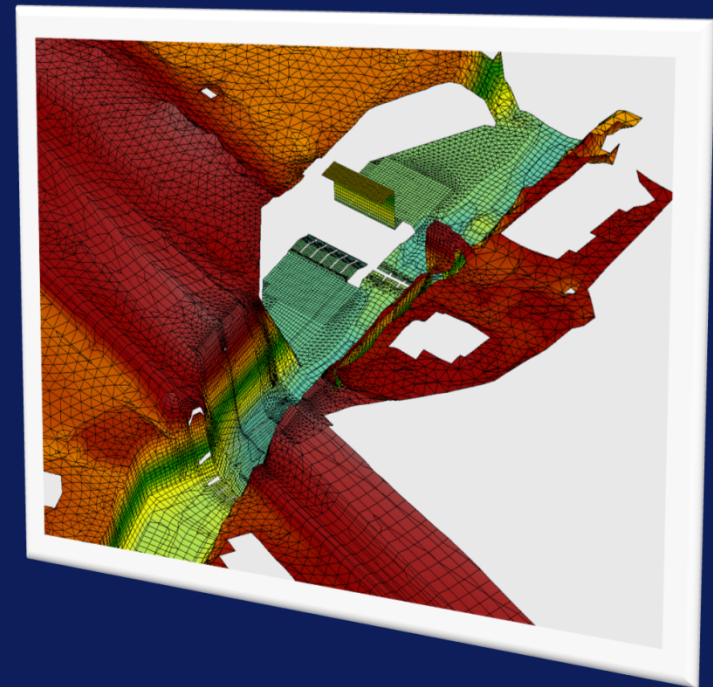
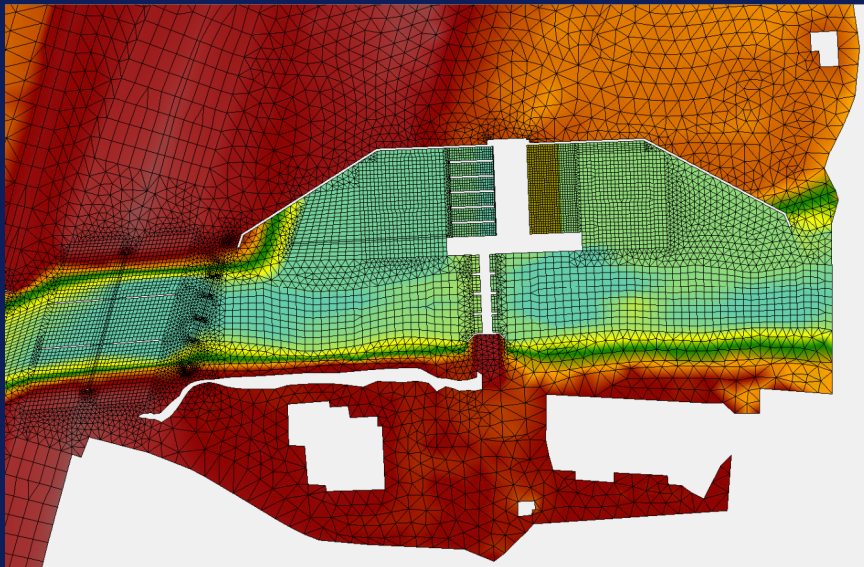


C-9 Canal and S-29 Gates

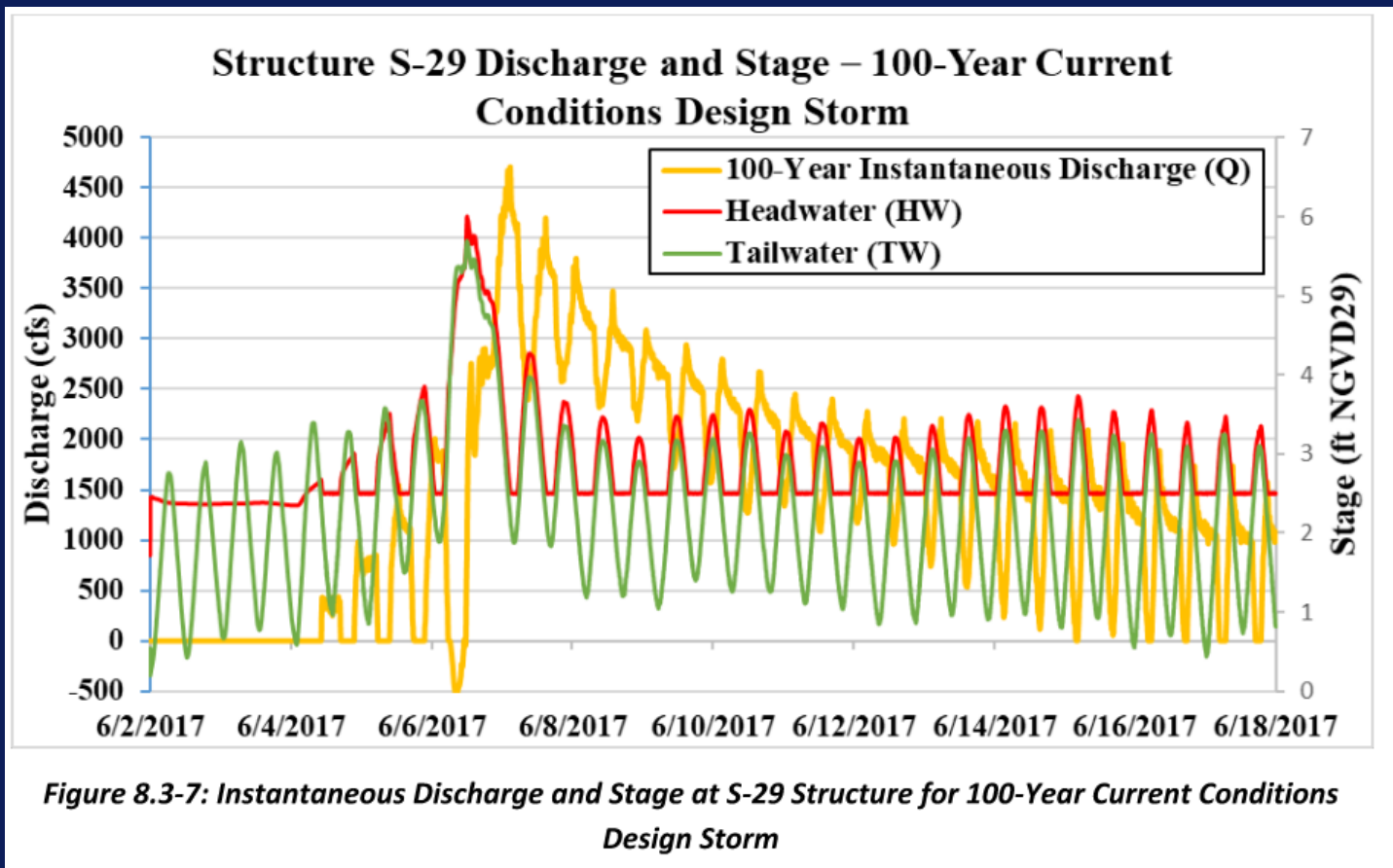


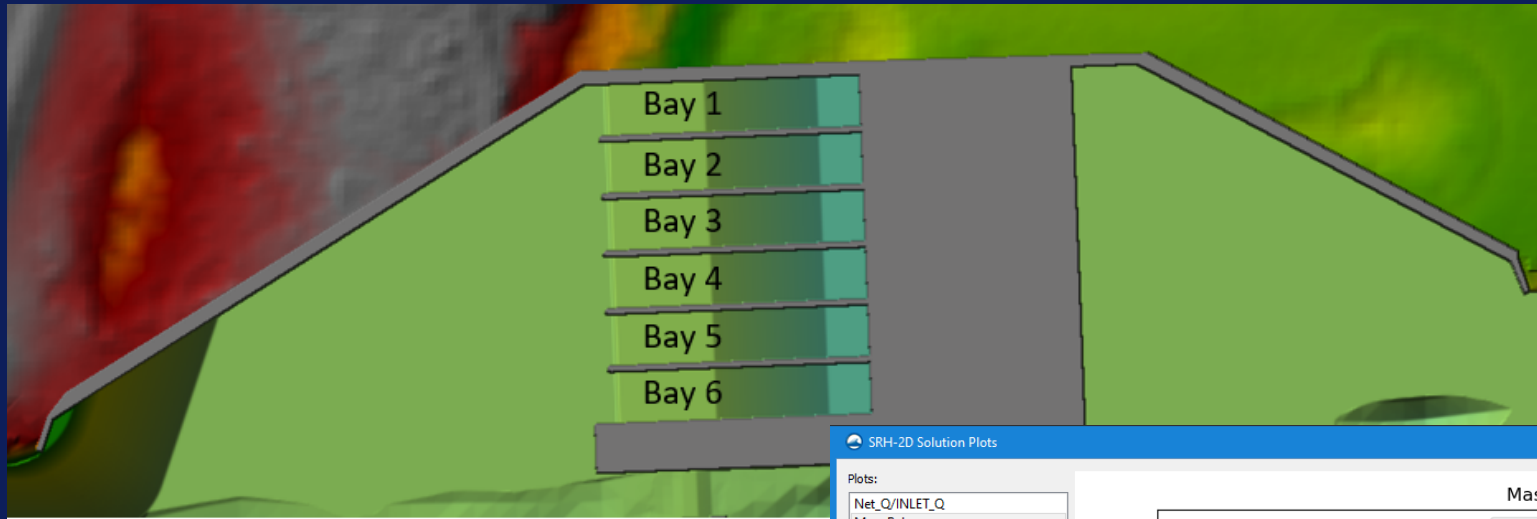
SRH-2D Model Documentation

- Holes in mesh
- 0.5-ft to 25-ft

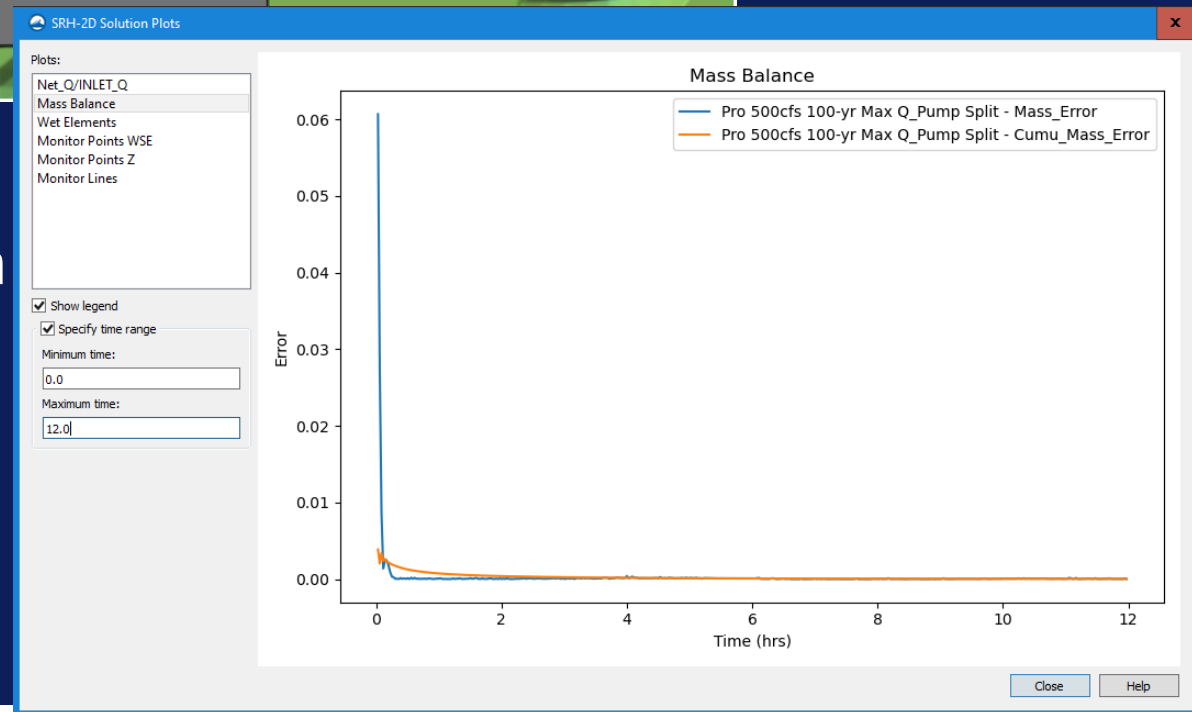


Inflow Hydrographs?





- Observe Results in Each Bay
- Stability



SRH-2D Results – Discharge Magnitude

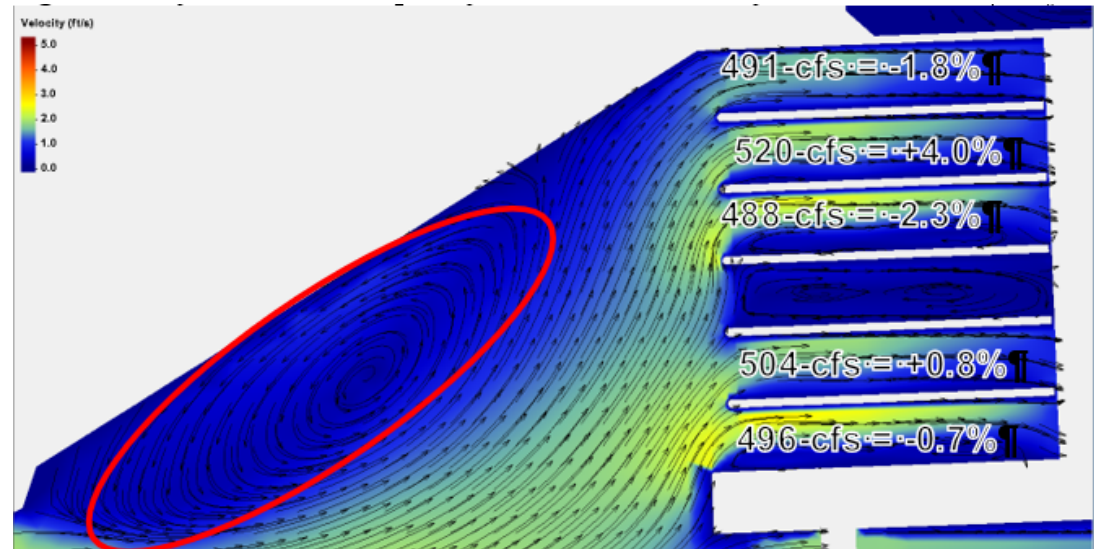


Figure 20: Flow patterns at 100-year maximum discharge

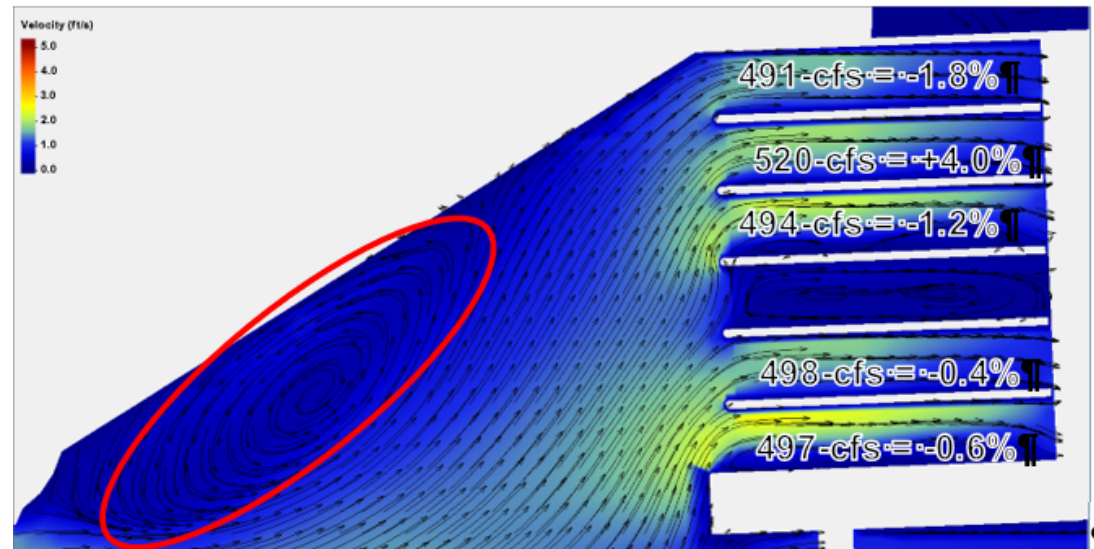


Figure 21: Flow patterns at 25-year maximum discharge

SRH-2D Results – Headwater & Discharge

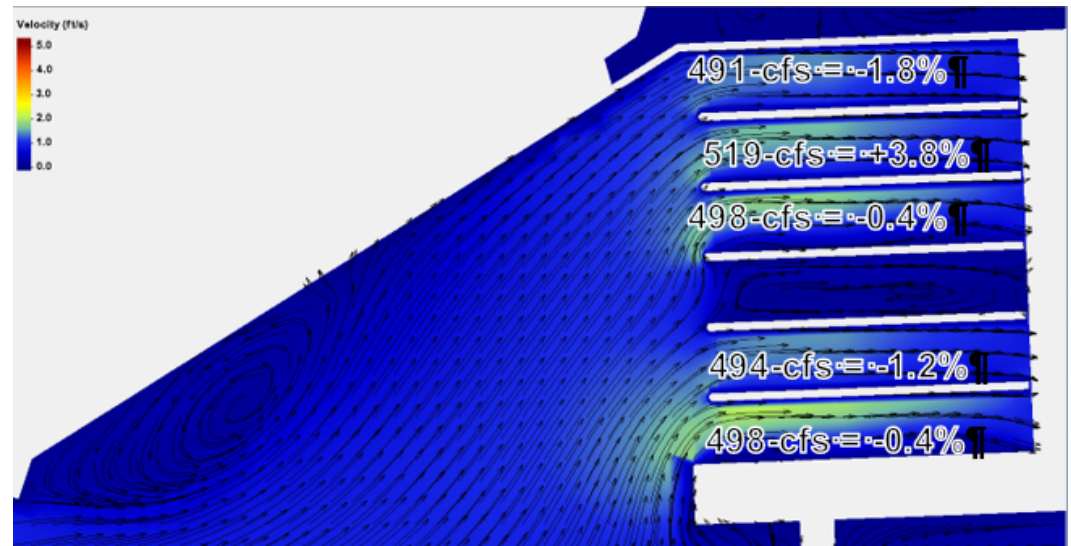


Figure 24: Flow patterns for 100-year maximum headwater

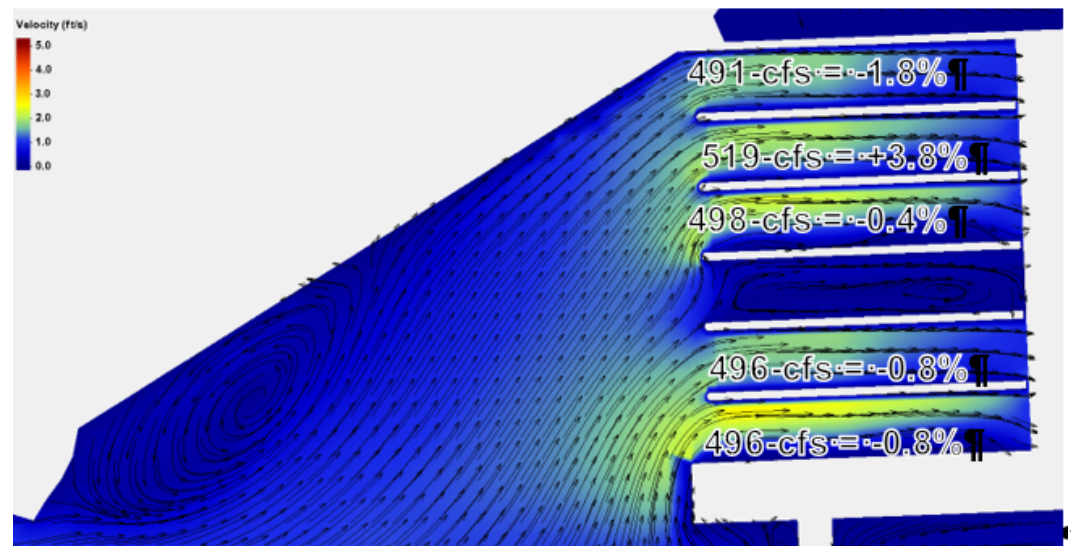
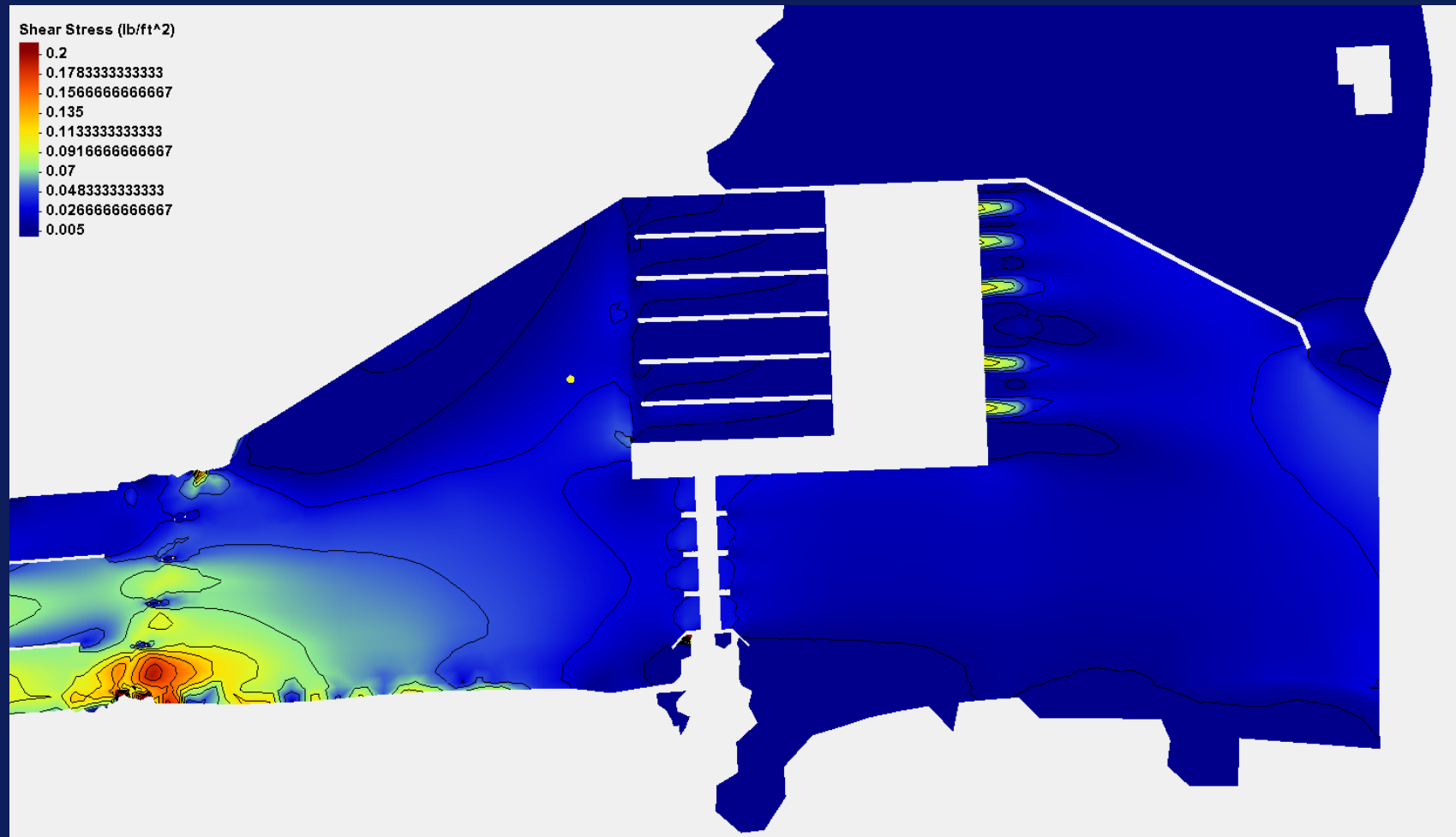


Figure 25: Flow patterns for 5-year maximum headwater

Shear Stress



Water Surface

