

From Blueprint to Permit: Stormwater Compliance in Primrose Farm Expansion

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Agenda

- » Background
 - Proposed site improvements
 - Existing stormwater facilities
 - Stormwater management design criteria
- » Stormwater Modeling Approach
 - Model setup, inputs and assumptions
- » Stormwater Modeling Results
 - Hydraulic model results summary
 - Stormwater mitigation management best practices (BPMs)
- » Summary



Background



Proposed Site Improvements

- » St. Charles Park District is proposing the following:
 - Additional building
 - Parking lot
 - Access roads on site
 - Storm sewer and water service utilities
 - Detention BMPs
 - Sidewalks

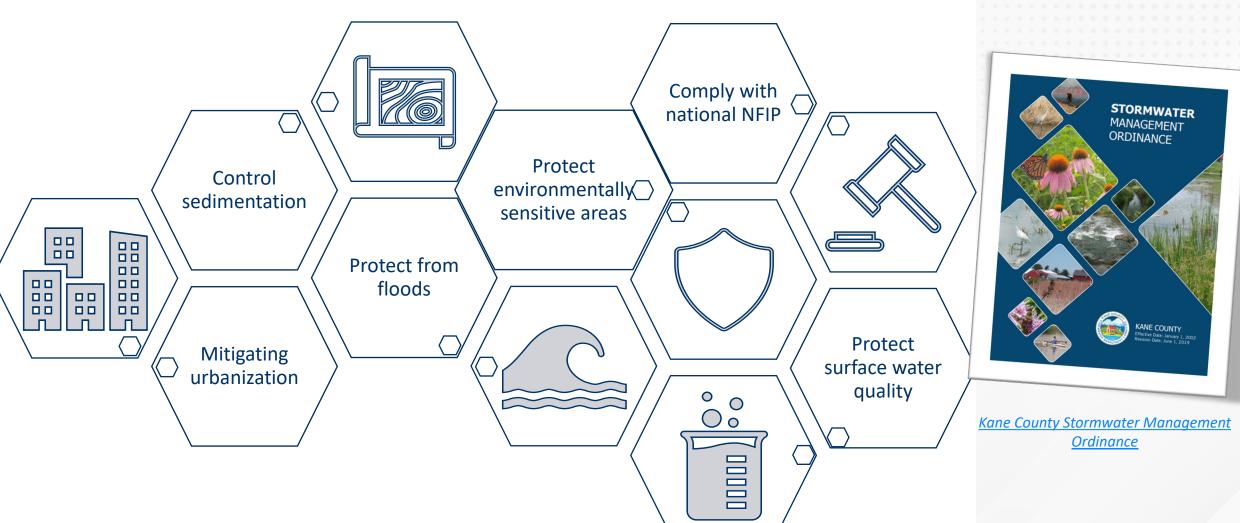




Site Location (Google Earth)



Why a Stormwater Management Permit?





Stormwater Management Design Criteria

- » Proposed redevelopments will add ~12,400 square feet of impervious area
 - Subject to Stormwater
 Mitigation/BMP requirements
 - Category I

	Development Category	for Devel New Imp	ervious Area opment or Net ervious Area velopment	Detention Storage Facility (Section)	Stormwater Mitigation / BMP (Section)	Watershed Benefit Measure ¹ (Section)	Fee-in- Lieu³ (Section)
	Development or Redevelopment	< 5,000 s	•		X ² (9-107 C)		A (9-85)
		sq.ft.	ft. – 24,999		(9-107.C)		A (9-85)
		< 1% Site		X (9-84)	(9-107.D)	O (9-108)	A (9-85)
		≥ 25,000 > 1% Site	sq.ft. AND	(9-84)	(9-107 D)		A (9-85)
Development Category	New Impervious Area for Development or Net		Stormwater Mitigation / BMP		Fee-in- Lieu ²	O (9-108)	A (9-85)
	New Impervious for Redevelopm		Category I (Section)	Category II (Section)	(Section)		Α
Development or Redevelopment	< 5,000 sq.ft.		X ¹ (9-107.C)		A (9-85)		(9-85)
	5,000 sq.ft. – 24,999 sq.ft.		(9-107.C)		A (9-85)	0	(9-85)
	≥ 25,000 sq.ft. AND < 1% Site area			X (9-107.D)	A (9-85)	(9-108)	A (9-85)
	≥ 25,000 sq.ft. AND ≥ 1% Site area			X (9-107.D)	A (9-85)	/	y
Linear Project (Trails/Roads)	> 1-acre in aggregate for roads and trails that are ≤ AASHTO max. width		X (9-107.C)		A (9-85)		
	> 1-acre in aggregate for roads and trails that are > AASHTO max. width			X (9-107.D)	A (9-85)		
Total Impervious Area > 50% Site area (for Sites < 1-acre)			X (9-107.C)		A (9-85)		
Hydrologically Disturbed Area > 3-acres				X (9-107.D)	A (9-85)		
X = Required; A = Allowed							

Stormwater Management Requirements (Kane County Stormwater Management Ordinance)



Stormwater Management Design Criteria

- » According to section 9-84 (*Detention Storage Facility Requirements*), the detention basin must:
 - Contain the 100-year storm volume
 - Allowable Release Rate less than or equal to 0.1 cfs/acre of development
 - Minimum diameter restrictor orifice of 4"
 - 1 ft freeboard above high-water elevation based on emergency overflow weir
- » According to section 9-107.C, Category I, BMPs must:
 - Provide volume reduction and water quality treatment
 - Volume = proposed impervious area and 1" rainfall event with no abstractions detained by the BMP



Impervious Area Mitigation





Two
Proposed
Rain
Gardens
BMPs

Existing
Stormwater
Facilities





Existing Stormwater Facilities

- » Three stormwater facilities
 - North detention basin
 - Existing depressional basin
 - South detention basin



Stormwater Facilities Locations (Kane County GIS Technologies)



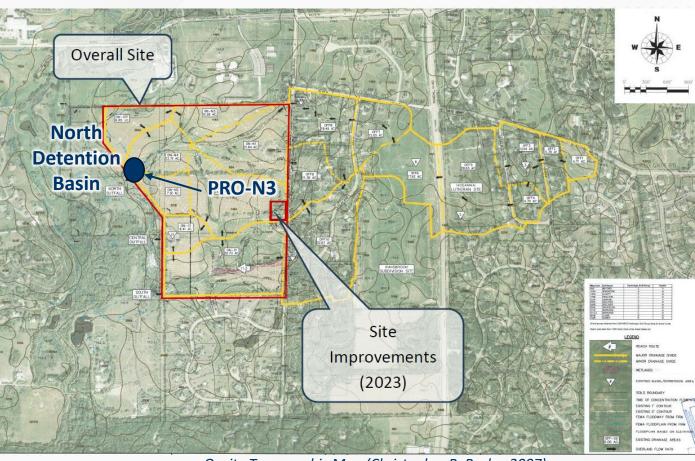
Site Drainage

» Site improvements are within sub-catchment PRO-N3

 Discharges to the north detention basin

Volume: 0.66 acre-ft

Overtop weir: 751.1 ft

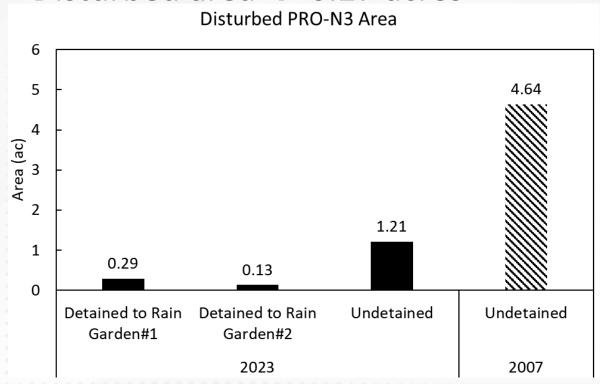


Onsite Topographic Map (Christopher B. Burke, 2007)



Sub-catchment PRO-N3

» Disturbed area → 6.27 acres







Stormwater Modeling Approach



Modeling Approach

- » Section 9-84C of Kane County Stormwater Management Ordinance:
 - Event hydrograph methods
 - Huff rainfall distribution
 - 24-hour duration
 - 100-year storm (1% probability of occurrence)

» The Hydrologic Modeling System (HEC-HMS)



Model Inputs and Outputs



Tributary Area and Land Use



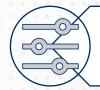
SCS* Curve Numbers & Unit Hydrographs



Rainfall Depths & Huff Distributions



Outlet Structure



Existing Storage Elevations

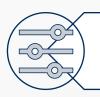


HEC-HMS

Runoff Volumes



Peak Storage



Peak Elevation



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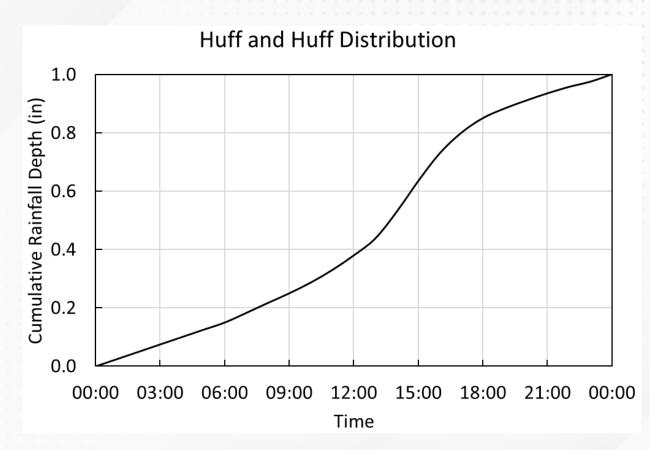
Subbasins Areas, Imperviousness, and Curve Numbers

Year	Category	Area (ac)	% Impervious	Curve Number
	Detained to Rain Garden#1	0.29	46%	84
2023	Detained to Rain Garden#2	0.13	38%	83
	Un-detained	1.21	33%	79
2007	Un-detained	4.64	82%	91



Rainfall and Huff Distribution

Year	Category	Total Rainfall* (in)	
2023	Detained to Rain Garden#1	8.57	
	Detained to Rain Garden#2		
	Un-detained		
2007	Un-detained	7.58	





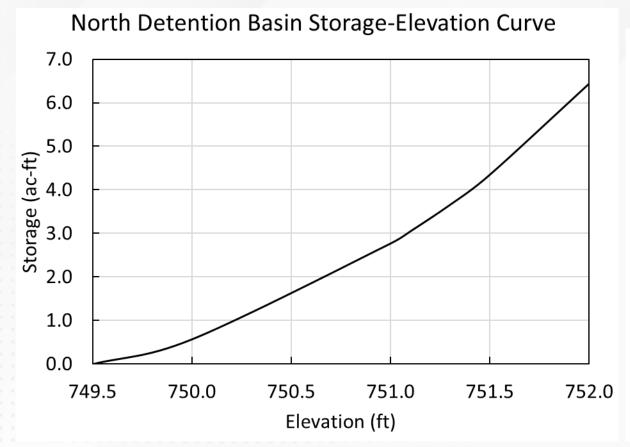
Outlet Structure

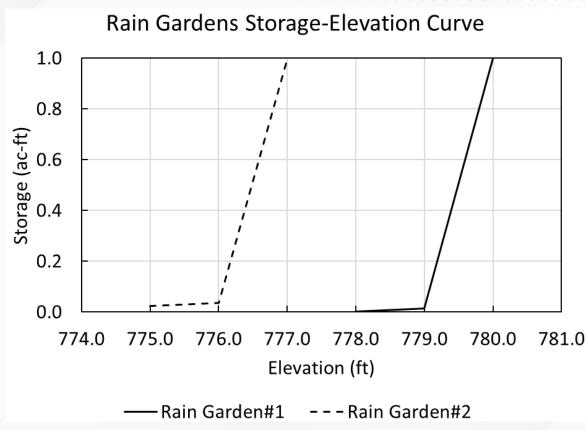
- » Spillway
 - Length → 400 ft
 - Elevation \rightarrow 751.1 ft

- » Orifice outlet
 - Center elevation → 749.5 ft
 - Diameter → 6"



Existing Storage Elevations







Stormwater Modeling Results



Modeling Results Summary

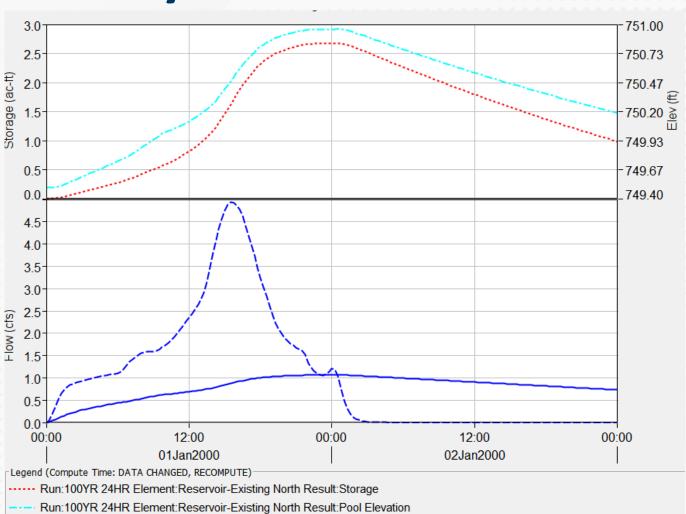
Kane County Stormwater Ordinance Criteria	Model Performance	Notes
Contain the 100-year storm volume	✓	The model was set to simulate the 100-year event and no overflow occurred.
Allowable Release Rate less than or equal to 0.1 cfs/acre of development plus the previously detained passthrough from the proposed rain gardens (1.087 cfs)	✓	The model peak discharge is 1.06 cfs.
Minimum diameter restrictor orifice of 4"	✓	The proposed orifice diameter is 5.8" versus the existing 6".
BMP volume = proposed impervious area and 1" rainfall event with no abstractions detained by the BMP		The required storage is 0.023 ac-ft, and the available storage is 0.13 ac-ft.

^{*}The available freeboard is 0.87 ft above the spillway high-water elevation (0.13 ft above the existing top of berm).



Modeling Results Summary

Result	Value	Unit
Peak Inflow	4.938	cfs
Peak Discharge	1.063	cfs
Peak Storage	2.677	ac-ft
Peak Elevation With Proposed Condition	750.96	ft



Run:100YR 24HR Element:Reservoir-Existing North Result:Outflow
 Run:100YR 24HR Element:Reservoir-Existing North Result:Combined Inflow



Questions?

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